Chapter 22:

Mitigation

A. INTRODUCTION

The preceding chapters of this environmental impact statement (EIS) discuss the potential for significant adverse impacts to result from the proposed Cornell NYC Tech project. Where such potential impacts have been identified—in the areas of historic and cultural resources, transportation (i.e., traffic, transit [bus line haul], and pedestrian conditions), and construction (i.e., construction-period transportation and noise impacts on open space)—measures are examined to minimize or eliminate the anticipated impacts to the fullest extent practicable. These mitigation measures are discussed below.

Areas in which the proposed project would result in significant adverse impacts that cannot be fully mitigated through reasonably practicable measures are discussed in Chapter 23, "Unavoidable Adverse Impacts."

In addition, this chapter analyzes the potential effects of the proposed traffic mitigation measures on pedestrian conditions.

B. HISTORIC AND CULTURAL RESOURCES

As discussed in Chapter 7, "Historic and Cultural Resources," the Goldwater Hospital complex has been determined eligible for listing on the State/National Registers of Historic Places (S/NR-eligible). The proposed project would demolish the Goldwater Hospital complex, which would constitute a significant adverse impact on this architectural resource. Cornell is consulting Measures to partially mitigate significant adverse impacts to the Goldwater Hospital complex would be implemented by Cornell in consultation with the New York State Office of Parks, Recreation and Historic Preservation (OPRHP) and the Landmarks Preservation Commission (LPC) regarding appropriate measures to partially mitigate the significant adverse impact. These measures, which would include the preservation of the Works Progress Administration (WPA) murals to the extent practicable, are being developed and will be implemented by Corneall, as set forth in a Letter of Resolution (LOR) by among Cornell, OPRHP, LPC, and the Roosevelt Island Operating Corporation (RIOC).

Mitigation measures include the following:

- 1. <u>Preparation of Historic American Buildings Survey (HABS) Level II documentation of the</u> <u>Goldwater Hospital complex, which would include photographic documentation, historic</u> <u>plans, and an accompanying historical narrative.</u>
- <u>Cornell has investigated the locations and conditions of the murals that were commissioned</u> for Goldwater Hospital as part of the Federal Art Project (FAP) of the WPA. To date, <u>Cornell has confirmed that four "abstraction" murals (works by Bolotowsky, Swinden,</u> <u>Rugolo, and Chanase) were installed in Goldwater Hospital, but that three of these have</u>

been painted over. Only Ilya Bolotowsky's "Abstraction" has been previously conserved and is currently visible.

Cornell has also investigated four additional murals (works by Goldman, Haupt, and two by Browne) and determined that they are not present in Goldwater Hospital. These murals were identified by the New York City Public Design Commission as having been commissioned, but there is no record of their installation. The investigations, conducted by EverGreene Architectural Arts and meeting the American Institute for Conservation of Historic and Artistic Works (AIC) standards, did not confirm the presence of these four murals at Goldwater Hospital.

- a. <u>Cornell would prepare a report on the findings of the investigations. A copy of the report shall be provided to OPRHP and LPC for review and comment.</u>
- b. <u>Cornell would, in consultation with OPRHP and LPC, develop and implement</u> <u>appropriate measures to remove and restore the four extant WPA murals to the</u> <u>extent practicable. Cornell would then promptly deliver all removed and restored</u> <u>WPA artwork to appropriate repositories, as identified in consultation with OPRHP</u> <u>and LPC.</u>
- c. <u>In consultation with OPRHP and LPC, Cornell would develop a digital media</u> <u>display about the murals, including information obtained through Cornell's</u> <u>investigations of the murals. The digital media display shall be submitted to OPRHP</u> <u>and LPC at the preliminary and pre-final stages for OPRHP and LPC comment. The</u> <u>location and management of the digital exhibit would be established through</u> <u>ongoing consultation with OPRHP and LPC.</u>
- 3. Cornell would develop and install one or more plaques or historic markers on the new academic campus that would provide information and a photograph describing and illustrating the history of the site, the Goldwater Hospital, and the WPA murals. Design for the interpretive materials shall be submitted to OPRHP and LPC at the preliminary and prefinal stages of development for OPRHP and LPC comment.

C. TRANSPORTATION

TRAFFIC

As discussed in Chapter 14, "Transportation," the proposed project would result in significant adverse traffic impacts at a number of locations in the traffic study area. This section describes the mitigation measures that could eliminate significant impacts. **Tables 22-1a** and **22-1b** summarize the significant adverse traffic impacts and identify if they could be fully or partially mitigated with the implementation of traffic improvement measures, or could not be mitigated.

	Traffic	Impact Mitigation	on Summary
Intersections	AM Peak Hour	Midday Peak Hour	PM Peak Hour
No significant impact	7 <u>9</u>	10 <u>11</u>	10
Impact could be fully mitigated	6 <u>5</u>	4 <u>3</u>	4
Impact could be partially mitigated	0	0	0
Unmitigated impact	<u> </u>	0	0

Table 22-1a Phase 1—2018 Analysis Year (2018 With Action Condition) Traffic Impact Mitigation Summary

	Table 22-1b
Full Build—2038	Analysis Year (2038 With Action Condition)
	Traffic Impact Mitigation Summary
	PM Peak

			PM Peak
Intersections	AM Peak Hour	Midday Peak Hour	Hour
No significant impact	4 <u>5</u>	7	3
Impact could be fully mitigated	5 6	3 5	7 <u>8</u>
Impact could be partially mitigated	0 <u>1</u>	0	0 <u>1</u>
Unmitigated impact	5 2	4 <u>2</u>	4 <u>2</u>

Details of the intersection capacity analyses and all traffic mitigation measures (e.g., signal timing changes, parking regulation changes, lane reconfigurations, etc.) are summarized in the level of service (LOS) tables presented in **Table 22-2** and **Table 22-3** at the end of the "Traffic" section.

The overall finding of the traffic mitigation analysis is that all <u>six</u> intersections under the 2018 With Action condition and <u>eight of the 11 intersections under</u> the 2038 With Action condition that would experience impacts could be fully mitigated with readily implementable traffic improvement measures, including signal timing and phasing changes, new traffic signals, parking regulation changes to gain or widen a travel lane at key intersections, and lane restriping. <u>One additional intersection under the 2038 With Action could be partially mitigated by adjusting the traffic signal timing</u>. These measures represent some of the standard traffic capacity improvements that are typically implemented by the New York City Department of Transportation (NYCDOT). Additional review of potential mitigation measures that may fully or partially mitigate other significant impact locations that are identified as unmitigatable in this Draft EIS will be undertaken for the Final EIS.

The following sections describe the potential mitigation measures in detail.

PHASE 1-2018 ANALYSIS YEAR (2018 WITH ACTION CONDITION)

As shown in **Table 22-1a**, in the weekday AM peak hour, seven <u>five</u> of the 14 intersections would be impacted and could be fully mitigated with the exception of one intersection; in the weekday midday peak hour, four <u>three</u> intersections would be impacted and could be fully mitigated; and in the weekday PM peak hour, four intersections would be impacted and could be fully mitigated.

Traffic mitigation measures needed for each intersection are described below; details of signal timing modifications are summarized in **Table 22-2a** and **Table 22-2b**.

Roosevelt Island Bridge/36th Avenue and Vernon Boulevard

Impacts on the northbound Vernon Boulevard shared left-turn/through/right-turn lane movement would occur during all three peak hours. Impacts on the southbound Vernon Boulevard shared left-turn/through/right-turn movement would occur during the AM peak hour. Both conditions the weekday AM and PM peak hours and could be mitigated by modifying the signal timing.

36th Avenue and 21st Street

Impacts on the eastbound 36th Avenue shared left-turn/through/right-turn movement would occur during the AM and midday peak hours. Impacts on the westbound 36th Avenue shared left-turn/through/right-turn movement would occur during the AM peak hour. Both conditions could be mitigated by modifying the signal timing.

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Broadway and 21st Street

Impacts on the eastbound Broadway shared left-turn/through/right-turn movement would occur during the AM, midday, and PM peak hours. Impacts would be experienced during the same peak hours and in the westbound direction for the same movements during the midday and PM peak hours. Both conditions could be mitigated by modifying the signal timing.

41st Avenue and Vernon Boulevard

Impacts on the northbound Vernon Boulevard shared through/right-turn movement would occur during the PM peak hour. Impacts on the southbound Vernon Boulevard shared left-turn/through movement would occur during the AM peak hour. Both conditions could be mitigated by modifying the signal timing.

Broadway and Vernon Boulevard/11th Street

Impacts on the westbound Broadway shared left turn/through/right turn movement and southbound shared left turn/through/right turn movement would occur during the AM peak hour. These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures will be undertaken for the Final EIS that may fully or partially mitigate these significant impacts.

Astoria Boulevard/27th Avenue/Newtown Avenue and 21st Street

Impacts on the northbound 21st Street shared left-turn/through/right-turn movement would occur during the midday and PM peak hours. Impacts on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM and midday peak hours. Both conditions could be mitigated by modifying the signal timing-and signal phasing to allow an eastbound/westbound exclusive left turn phase.

Hoyt Avenue South and 21st Street

Impacts on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM peak hour and could be mitigated by modifying the signal timing and allowing through movements and left turns from the 11-foot wide exclusive left-turn lane on the eastbound approach of Hoyt Avenue South.

FULL BUILD-2038 ANALYSIS YEAR (2038 WITH ACTION CONDITION)

As shown in **Table 22-1b**, in the weekday AM peak hour, 10 <u>nine</u> of the 14 intersections would be impacted, five <u>six</u> of which could be fully mitigated, <u>one could be partially mitigated</u>, and the other five <u>two</u> could not be mitigated; in the weekday midday peak hour, seven intersections would be impacted, three <u>five</u> of which could be fully mitigated and four <u>two</u> could not be mitigated; and in the weekday PM peak hour, 11 intersections would be impacted, seven <u>eight</u> of which could be fully mitigated, and four <u>two</u> could not be mitigated.

Traffic mitigation measures needed for each intersection are described below; details of signal timing modifications are summarized **Table 22-3a** and **Table 22-3b**.

West Road and Main Street

Impacts on the eastbound West Road shared left-turn/right-turn movement would occur during the PM peak hour and could be mitigated by installing a traffic signal. Because installing a single traffic signal would not control all the traffic movements at this triangle-shaped

intersection, and it is desirable to eliminate the observed, illegal northbound movements occurring against southbound traffic on the north leg of the triangle, it is recommended to "normalize" this intersection to eliminate superfluous vehicular turning conflicts and pedestrian conflicts so that the south leg no longer carries vehicular traffic and is "pedestrianized." This improvement would allow vehicular and pedestrian movements to occur at the intersection of West Road and Main Street and be under the control of a single new traffic signal. This would also provide unrestricted pedestrian access to the existing triangle from west of Main Street and east of West Road. It should be noted that this would divert existing trips (mainly passenger vehicles) that use the traffic triangle as a U-turn to one block south to the traffic circle at East Road; about 80 vehicles per hour in the AM peak hour and about 40 vehicles per hour in the midday and PM peak hours would be diverted in the 2038 Full Build condition. An analysis of the Main Street at East Road/West Road traffic circle with this traffic diversion is included in the detailed level of service summary tables at the end of the chapter. The mitigation currently identified has been determined to be feasible. will be further reviewed for the Final EIS by RIOC and NYCDOT. If the mitigation measures are not feasible, and no other measures are available to fully mitigate the impacts, the intersection may be identified as partially mitigated or unmitigatable in the Final EIS.

Roosevelt Island Bridge Ramp and Main Street

Impacts on the westbound Roosevelt Island Bridge Ramp shared left-turn/right-turn movement would occur during the AM peak hour. Impacts on the northbound Main Street right-turn lane would occur during the PM peak hour. Both conditions could be mitigated by installing a traffic signal. The mitigation currently identified <u>has been determined to be feasible.</u> will be further reviewed for the Final EIS by RIOC and NYCDOT. If the mitigation measures are not feasible, and no other measures are available to fully mitigate the impacts, the intersection may be identified as partially mitigated or unmitigatable in the Final EIS.

Roosevelt Island Bridge/36th Avenue and Vernon Boulevard

Impacts on the eastbound Roosevelt Island Bridge shared through/right-turn movement would occur during the PM peak hour. Impacts on the northbound Vernon Boulevard shared left-turn/through/right-turn movement would occur during all peak hours. In the southbound direction of Vernon Boulevard, the shared left-turn/through/right-turn movement would experience impacts during the AM and PM peak hours. These impacts <u>could be fully mitigated</u> for the midday peak hour and partially mitigated for the AM and PM peak hours by modifying the traffic signal cycle from 60 seconds to 90 seconds.-are currently identified as unmitigatable, but additional review of potential mitigation measures will be undertaken for the Final EIS that may fully or partially mitigate these significant impacts.

36th Avenue and 21st Street

Impacts were identified on the following approaches:

- The eastbound 36th Avenue shared left-turn/through/right-turn movement during all peak hours,
- The westbound 36th Avenue shared left-turn/through/right-turn movement during all peak hours,
- The northbound 21st Street shared left-turn/through/right-turn movement during the midday peak hour, and,

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• The southbound 21st Street shared left-turn/through/right-turn movement during the AM peak hour.

Overall, the impacts could be mitigated by modifying the signal timing and making the following modifications:

- Shifting the eastbound approach centerline six feet to the north and restriping the approach from one 25-foot wide travel lane to one 11-foot wide exclusive left-turn lane and one 20-foot wide shared through/right-turn lane, with parking for a distance of 200 feet back from the intersection, and
- Shifting the westbound approach centerline six feet to the south and restriping the approach from one 25-foot wide travel lane to one 11-foot wide exclusive left-turn lane and one 20-foot wide shared through/right-turn lane with parking for a distance of 125 feet back from the intersection.

Broadway and 21st Street

Impacts were identified on the following approaches:

- The eastbound Broadway shared left-turn/through/right-turn movement during all peak hours,
- The westbound Broadway shared left-turn/through/right-turn movement during all peak hours,
- The northbound 21st Street shared left-turn/through/right-turn movement during the PM peak hour, and,
- The southbound 21st Street shared left-turn/through/right-turn movement during the AM peak hour.
- These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate these significant impacts will be undertaken for the Final EIS.

These impacts could be fully mitigated for all peak hours with the following measures:

- <u>Prohibit parking along the eastbound approach for a distance of 200 feet from the intersection</u> (a loss of approximately five parking spaces), and along the eastbound receiving side for a distance of 250 feet from the intersection (a loss of approximately three parking spaces).
- <u>Shift the eastbound approach centerline three feet to the north and restripe the approach from one 22-foot wide travel lane with parking to one 10-foot wide exclusive left-turn lane and one 15-foot wide shared through/right-turn lane for a distance of 200 feet back from the intersection.</u>
- <u>Shift the westbound approach centerline seven feet to the south and restripe the approach from one 22-foot wide travel lane with parking to one 10-foot wide exclusive left-turn lane and one 19-foot wide shared through/right-turn lane with parking for a distance of 250 feet back from the intersection, and</u>
- Modify the signal timing.

36th Avenue and 31st Street

Impacts on the eastbound 36th Avenue shared left-turn/through/right-turn movement would occur during the midday and PM peak hours and could be mitigated by modifying the signal timing.

41st Avenue and Vernon Boulevard

Impacts on the northbound Vernon Boulevard shared through/right-turn movement would occur during the PM peak hour. Impacts on the southbound Vernon Boulevard shared left-turn/through movement would occur during the AM and PM peak hour. Both conditions could be mitigated by modifying the signal timing.

30th Avenue and 21st Street

An impact on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM peak hour. This impact is currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate the significant impact will be undertaken for the Final EIS.

Broadway and Vernon Boulevard/11th Street

Impacts on the westbound Broadway shared left-turn/through/right-turn movement would occur during all peak hours. Impacts on the southbound Vernon Boulevard shared left-turn/through/right-turn movement would occur during the AM and PM peak hours. These impacts <u>could not be</u> <u>mitigated.</u> are currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate these significant impacts will be undertaken for the Final EIS.

Astoria Boulevard/27th Avenue/Newtown Avenue and 21st Street

Impacts were identified on the following approaches:

- The eastbound Astoria Boulevard shared through/right-turn lane during the AM and PM peak hours,
- The westbound Astoria Boulevard shared through/right-turn lane during the PM peak hour,
- The northbound 21st Street shared left-turn/through/right-turn movement during the AM and midday peak hours,
- The northbound 21st Street shared through/right-turn lane during the PM peak hour, and
- The southbound 21st Street shared left-turn/through/right-turn movement during all peak hours.

Overall, the intersection could be mitigated by modifying the signal timing and signal phasing to allow an eastbound/westbound exclusive left-turn phase. with the following measures:

- <u>Prohibit parking along the southbound approach for a distance of 100 feet from the intersection (a loss of approximately four parking spaces).</u>
- Restripe the northbound approach from one 11-ft shared left-through lane and one 20-ft shared through-right lane with parking to one 11-ft shared left-through lane, one 10-ft travel lane, and one 10-ft parking lane which would serve as a right turn lane during the weekday PM peak period. Prohibit parking on the northbound approach for 100 feet from the intersection (a loss of approximately three parking spaces).
- Shift the southbound approach centerline two feet to the east and restripe the approach from one 11-foot wide shared left-turn/through lane and one 19-foot wide shared through/right-turn lane with parking to one 11-foot wide shared left-turn/through lane, one 10-foot wide travel lane, and one 11-foot wide parking lane which would serve as a right turn lane during the weekday AM and PM peak periods, and
- <u>Modify the signal timing.</u>

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Hoyt Avenue North and 21st Street

Impacts were identified on the following approaches:

- The westbound Hoyt Avenue North left-turn lane during all peak hours,
- The northbound 21st Street through lane during the AM and PM peak hours, and
- The southbound 21st Street shared through/right-turn lane during the AM and PM peak hours.
- These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate these significant impacts will be undertaken for the Final EIS.

These impacts could not be mitigated.

Hoyt Avenue South and 21st Street

Impacts on the northbound 21st Street shared left-turn/through/right-turn movement would occur during the AM and PM peak hours. Impacts on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM and PM peak hours. Both conditions could be mitigated by modifying the signal timing and allowing through movements and left turns from the 11-foot wide exclusive left-turn lane on the eastbound approach of Hoyt Avenue South.

CONCLUSION

The overall finding of the traffic mitigation analysis is that all but one of the 14 intersections analyzed under the 2018 With Action condition and all but five three under the 2038 With Action condition would either not be significantly impacted or could be fully mitigated with readily implementable traffic improvement measures, including signal timing and phasing changes, new traffic signals, parking regulation changes to gain or widen a travel lane at key intersections, and lane restriping. One additional intersection under the 2038 With Action could be partially mitigated by adjusting the traffic signal timing. Additional review of potential mitigation measures that may fully or partially mitigate the significant impacts that are identified as unmitigatable will be undertaken for the Final EIS.

The implementation of these measures would result in the loss of approximately 12 to 18 parking spaces during various times of the day and days of the week. Broadway would lose up to eight parking spaces between Vernon Boulevard and 23rd Street and 21st Street would lose up to seven parking spaces between Astoria Boulevard and 24th Road. No designated truck loading/unloading zones or bus layover spaces would be affected by the proposed parking modifications for mitigation. If it is determined that on-street parking should be retained at locations where such mitigation was assumed, additional unmitigated traffic impacts would result.

Table 22-2a 2018 No Action, With Action, and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

		1							(UII)					ections)
			No	Action			With	Action		V	Vith N	litigatio	n	Midaadaa
Intersection	Approach	Mad	v/c	Control	1.00	Mad	v/c	Control	LOS	N/1-14	v/c	Control	LOS	Mitigation
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	105	M∨t.	V/C	Delay	LUS	Measure
				AM Pe	ак ног	Ir								
1. EAST/WEST MAIN STREE			1					- 4	•	1		1	1	
West Road	EB	LT	-	7.1	A	LT	-	7.4	A	-	-	-	-	-Mitigation
Main Street	SB	LR	-	7.3	A	LR	-	8.3	A	-	-	-	-	not
	Overall Inters	ection	-	7.3	Α	-	-	8.3	Α	-	-	-	-	required.
2. WEST ROAD & MAIN STR			r						_	1	1		1	1
West Road	EB	LR	-	9.1	A	LR	-	10.4	В	-	-	-	-	
West Road (south of island)	EB	LR	-	11.3	В	LR	-	12.5	В	-	-	-	-	-Mitigation
Main Street	NB	LT	-	9.9	Α	LT	-	10.6	В	-	-	-	-	not
	SB	TR	-	9.3	A	TR	-	11.7	В	-	-	-	-	required.
	Overall Inters		-	9.8	Α	-	-	11.3	В	-	-	-	-	
3. ROOSEVELT ISLAND BR	DGE RAMP & I	AIN S	TREE	T										-
Roosevelt Island Bridge	WB	LR	_		в	LR	-	23.2	С	-	-	-	-	
Ramp				14.6					-					-Mitigation
	NB	Т	-	10.2	В	Т	-	10.7	В	-	-	-	-	not
Main Street		R	-	10.8	В	R	-	13.2	В	-	-	-	-	required.
	SB	LT	-	12.2	В	LT	-	13.5	В	-	-	-	-	requireu
	Overall Interse		-	12.8	В	-	-	18.0	С	-	-	-	-	
4. ROOSEVELT ISLAND BR	DGE & MOTOR	GATE	GARA	GE ENTI	RANCE	: / EX	T							
Roosevelt Island Bridge	EB	LT	-	8.4	Α	LT	I	8.8	А	-	-	-	-	-Mitigation
Motorgate Garage Exit	NB	LR	-	11.2	В	LR	I	11.9	В	-	-	-	-	not
	Overall Interse	ection	-	1.4	Α	-	-	1.4	Α	-	-	-	-	required.
				Midday F	Peak H	our								
1. EAST/WEST MAIN STREE	T & MAIN STR	EET												
West Road	EB	LT	-	7.6	Α	LT	-	7.9	А	-	-	-	-	-Mitigation
Main Street	SB	LR	-	7.3	Α	LR	-	8.3	Α	-	-	-	-	not
	Overall Inters	ection	-	7.4	Α	-	-	8.2	Α	-	-	-	-	required.
2. WEST ROAD & MAIN STR														
West Road	EB	LR	-	8.4	Α	LR	-	10.1	В	-	-	-	-	
West Road (south of island)	EB	LR	-	10.7	B	LR	-	11.9	В	-	-	-	-	-Mitigation
	NB	LT	-	9.2	A	LT	-	10.1	B	-	-	-	-	not
Main Street	SB	TR	-	8.6	A	TR	-	10.1	B	-	-	-	-	required.
	Overall Inters		-	9.2	A	-	-	10.6	B	-	-	_	_	requirea
3. ROOSEVELT ISLAND BR				-	~	-	-	10.0	D	-	-	-	-	
	DGE KANIF & I		IKEE		1				1	1	1	1		
Roosevelt Island Bridge Ramp	WB	LR	-	10.1	В	LR	-	13.7	В					
Kamp	NB	т	-	9.2	А	т	-	9.8	А	-	-	-	-	-Mitigation
	IND	R	-			R	-	9.0	B	-	-	-	-	not
										-	-	-	-	no ou tino d
Main Street	00			9.0	A									required.
Main Street	SB	LT	-	10.5	В	к LT	-	11.7	В	-	-	-	-	requirea.
	Overall Inters	LT ection	-	10.5 9.8	B A	LT -	-			-	-	-	-	required.
4. ROOSEVELT ISLAND BR	Overall Inters	LT ection GATE	-	10.5 9.8 GE ENTI	B A RANCE	LT - / EX	-	11.7 12.2	В В				-	
4. ROOSEVELT ISLAND BRI Roosevelt Island Bridge	Overall Interse DGE & MOTOR EB	LT ection GATE LT	- GARA -	10.5 9.8 GE ENTI 7.7	B A RANCE A	LT - / EX	- - T -	11.7 12.2 7.9	В В А	-	-	-	-	-Mitigation
4. ROOSEVELT ISLAND BR	Overall Interse DGE & MOTOR EB NB	LT Ection GATE LT LR	-	10.5 9.8 GE ENTI 7.7 9.9	B A RANCE A A	LT - / EX LT LR	-	11.7 12.2 7.9 10.6	В В А В	-	-		-	-Mitigation not
4. ROOSEVELT ISLAND BRI Roosevelt Island Bridge	Overall Interse DGE & MOTOR EB	LT Ection GATE LT LR	- GARA -	10.5 9.8 GE ENTI 7.7 9.9 0.9	B A A A A A	LT - / EX LT LR -	- - T -	11.7 12.2 7.9	В В А	-	-	-	-	-Mitigation
4. ROOSEVELT ISLAND BR Roosevelt Island Bridge Motorgate Garage Exit	Overall Interse DGE & MOTOR EB NB Overall Interse	LT CGATE LT LR ection	- GARA -	10.5 9.8 GE ENTI 7.7 9.9	B A A A A A	LT - / EX LT LR -	- - T -	11.7 12.2 7.9 10.6	В В А В	-	-	-	-	-Mitigation not
4. ROOSEVELT ISLAND BR Roosevelt Island Bridge Motorgate Garage Exit	Overall Interse DGE & MOTOR EB NB Overall Interse	LT CGATE LT LR ection	- GARA -	10.5 9.8 GE ENTI 7.7 9.9 0.9	B A A A A A	LT - / EX LT LR -	- - T -	11.7 12.2 7.9 10.6	В В А В	-	-	-	-	-Mitigation not
4. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit 1. EAST/WEST MAIN STREE	Overall Interse DGE & MOTOR EB NB Overall Interse T & MAIN STR EB	LT CGATE LT LR ection	- GARA -	10.5 9.8 GE ENTI 7.7 9.9 0.9	B A A A A A	LT - / EX LT LR -	- - T -	11.7 12.2 7.9 10.6	В В А В	-	-	-	-	-Mitigation not required.
4. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit 1. EAST/WEST MAIN STREE West Road	Overall Interse DGE & MOTOR EB NB Overall Interse T & MAIN STR	LT CATE CATE LT LR CET	- GARA -	10.5 9.8 GE ENTI 7.7 9.9 0.9 PM Pe	B A A A A A ak Hou	LT - - - - - -	- - T -	11.7 12.2 7.9 10.6 0.7	В В А В А	-	-	-	-	-Mitigation not required.
4. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit 1. EAST/WEST MAIN STREE West Road Main Street	Overall Interse DGE & MOTOR EB NB Overall Interse T & MAIN STR EB SB Overall Interse	LT CATE CATE LT LR Cotion EET LT LR	- GARA - - -	10.5 9.8 GE ENTI 7.7 9.9 0.9 PM Pe 7.4	B A A A A ak Hou A	LT - / EXI LT LR - ir	- - T - -	11.7 12.2 7.9 10.6 0.7 7.6	В В А В А	-		-	-	-Mitigation not required. -Mitigation
4. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit 1. EAST/WEST MAIN STREE West Road Main Street	Overall Interse DGE & MOTOR EB NB Overall Interse T & MAIN STR EB SB Overall Interse	LT CATE CATE LT LR Cotion EET LT LR	- GARA - - - -	10.5 9.8 GE ENTI 7.7 9.9 0.9 PM Pe 7.4 7.2	B A A A A ak Hou A A	LT - LT LR - r LT LR	- - T - - -	11.7 12.2 7.9 10.6 0.7 7.6 8.0	В В А В А А А	-	- - - -	- - - - -	-	-Mitigation not required. -Mitigation not
A. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit Anticology Stress MAIN STREE West Road Main Street West ROAD & MAIN STREE	Overall Interse DGE & MOTOR EB NB Overall Interse T & MAIN STR EB SB Overall Interse	LT CATE CATE LT LR Cotion EET LT LR	- GARA - - - -	10.5 9.8 GE ENTI 7.7 9.9 0.9 PM Pe 7.4 7.2	B A A A A ak Hou A A	LT - LT LR - r LT LR	- - T - - -	11.7 12.2 7.9 10.6 0.7 7.6 8.0	В В А В А А А	-	- - - -	- - - - -	-	-Mitigation not required. -Mitigation not
A. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit Anticological Stress Stress Vest Road Main Street West Road & MAIN STR West Road	Overall Interse DGE & MOTOR EB NB Overall Interse T & MAIN STR EB SB Overall Interse EET	LT GATE LT LR ection EET LT LR ection	- GARA - - - -	10.5 9.8 GE ENTI 7.7 9.9 0.9 PM Pe 7.4 7.2 7.3 8.7	B A RANCE A A ak Hou A A A A A	LT - LT LR - Ir LR LR -	- - T - - -	11.7 12.2 7.9 10.6 0.7 7.6 8.0 7.9 13.0	В В В А А А А А	-	- - - -	- - - - -	-	-Mitigation not required. -Mitigation not required.
A. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit Anternational Street Vest Road Main Street Vest Road West Road West Road West Road West Road West Road (south of island)	Overall Inters DGE & MOTOR EB NB Overall Inters EB SB Overall Inters EB EET EB EB	LT GATE LT LT LR ection EET LT LR ection	- GARA - - - - - - - - -	10.5 9.8 GE ENTI 7.7 9.9 0.9 PM Pe 7.4 7.2 7.3 8.7 10.6	B A A A A A A A A A A B	LT - /EXI LT LR - IT LR - LR LR LR	- - - - - -	11.7 12.2 7.9 10.6 0.7 7.6 8.0 7.9 13.0 11.4	В В А В А А А А В В В	- - - - - - - - -	- - - - - -	- - - - - -	- - - - - - - - - -	-Mitigation not required. -Mitigation not required.
A. ROOSEVELT ISLAND BRI Roosevelt Island Bridge Motorgate Garage Exit Automatic Street West Road Main Street West Road & MAIN STREE West Road	Overall Interso DGE & MOTOR EB NB Overall Interso EB SB Overall Interso EET EB	LT CGATE CGATE LT LR CCTON EET LT LR CCTON	- GARA - - - - - - - - - - -	10.5 9.8 GE ENTI 7.7 9.9 0.9 PM Pe 7.4 7.2 7.3 8.7	B A A A A A A A A A A A	LT - LT LR - IT LR - LR - LR	- T - - - - - -	11.7 12.2 7.9 10.6 0.7 7.6 8.0 7.9 13.0	В В А В А А А А А В	- - - - - - - - -	- - - - - - - - - -	- - - - - - - - - - - - -	- - - - - - - - - -	-Mitigation not required. -Mitigation not

Table 22-2a (cont'd) 2018 No Action, With Action, and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

		No Action With Action								W	/ith N	litigatio	n	
Intersection	Approach	Mvt.	v/c	Control Delay	LOS	Mvt.	v/c	Control Delay	LOS	Mvt.	v/c	Control Delay	LOS	Mitigation Measure
			PM	Peak Hou	ur (con	tinue	d)							
3. ROOSEVELT ISLAND BRI	DGE RAMP & N	IAIN S	TREE	Г										
Roosevelt Island Bridge Ramp	WB	LR	-	11.0	В	LR	-	13.9	В	-	-	-	-	
	NB	Т	-	9.6	Α	Т	-	10.1	В	-	-	-	-	-Mitigation
Main Street		R	-	9.6	Α	R	-	13.0	В	-	-	-	-	not required.
	SB	LT	-	14.2	В	LT	-	16.7	С	-	-	-	-	required.
	Overall Interse	ction	-	11.9	В	-	-	14.4	в	-	-	-	-	
4. ROOSEVELT ISLAND BRI	DGE & MOTOR	GATE	GARA	GE ENTR	RANCE	E / EXI	Т							
Roosevelt Island Bridge	EB	LT	-	7.9	Α	LT	-	8.1	Α	-	-	-	-	-Mitigation
Motorgate Garage Exit	NB	LR	-	12.5	В	LR	-	14.3	В	-	-	-	-	not
	Overall Interse	ction	-	1.0	Α	-	-	0.9	Α	-	-	-	-	required.
Notes: (1) Control delay is measured (2) Overall intersection V/C ra				//C ratio.										

Table 22-2b 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		No Action			With Action					With M	itigation			
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mitigation Measure
						AM Pea								
5. ROOSEVELT ISLAND		AVEN				EVARD								
Roosevelt Island Bridge	EB	L	0.29	13.0	В	L	0.34	13.8	В	L	<u>0.38</u>	<u>15.7</u>	B	
0.001		TR	0.59	16.9	В	TR	0.67	19.0	В	TR	0.73	22.6	<u>C</u>	-Modify signal timing:
36th Avenue	WB	LTR	0.37	13.7	В	LTR	0.44	14.7	B	<u>LTR</u>	<u>0.50</u>	<u>17.1</u>	B	shift 4 2 s green time
Vernon Boulevard	NB	LTR	1.12 0.92	75.0 20.4	E C	LTR	1.38 1.12	188.5 75.7	E E	LTR	1.00	29.1	<u>C</u>	from EB/WB phase to
			1.06	<u>20.4</u> 57.4	E		1.12	71.1	E*					NB/SB phase [EB/WB green time shifts from 25
	SB	LTR	0.89	23.2	Ċ	LTR	0.92	26.5	C	<u>LTR</u>	<u>0.85</u>	<u>19.0</u>	<u>B</u>	s to 21 23 s; NB/SB
	0		0.85	45.5	Ð		1.02	77.4	E		0.00	04.0	~	green time shifts from 25
	Overall Inters	ection	0.76	<u>19.2</u>	B	-	0.90	<u>34.8</u>	<u>C</u>	Ξ	<u>0.88</u>	<u>21.8</u>	<u>C</u>	s to 29 <u>27</u> s].
6. 36TH AVENUE & 21S	T STREET													
36th Avenue	EB	LTR	0.73	44.1	D	LTR	0.90	58.2	E*	LTR	0.83	49.0	D	-Modify signal timing:
	WB	LTR	0.91	48.0	D	LTR	0.97	55.6	E*	LTR	0.91	46.1	D	shift 2 s green time from
21st Street	NB	LTR	0.34	12.2	В	LTR	0.34	12.2	В	LTR	0.35	13.3	В	NB/SB phase to EB/WB phase [EB/WB green
	SB	LTR	0.98	28.9	С	LTR	1.00	33.4	С	LTR	1.03	43.4	D	time shifts from 37 s to 39
	Overall Inters	ection	0.96	29.6	с	-	0.99	35.0	D	-	0.99	38.8	D	s; NB/SB green time
			0.00		•		0.00		_		0.00		_	shifts from 73 s to 71 s].
7. BROADWAY & 21ST	STREET													
Broadway	EB	LTR	0.98	78.6	Е	LTR	1.00	82.6	F*	LTR	0.96	72.8	E	
Diodanay	20	2	<u>0.84</u>	<u>55.7</u>		2	<u>0.85</u>	<u>57.1</u>	E	2	0.00	72.0	_	
	WB	LTR	0.97	69.5	E	LTR	1.00	74.9	E*	LTR	0.95	63.6	E	-Mitigation not required.
			0.87 0.48	<u>54.6</u> 15.9	<u>D</u>		0.89 0.49	<u>56.5</u> 16.0	<u>E</u>					-Modify signal timing:
21st Street	NB	LTR	0.46	15.5	В	LTR	0.49	15.8	В	LTR	0.50	16.7	₿	shift 1 s green time from NB/SB phase to EB/WB
			0.99	32.7	-		1.01	<u>38.6</u>	Ð				_	phase [EB/WB green
	SB	LTR	0.95	27.9	С	LTR	0.97	29.7	Č	LTR	1.03	4 3.7	Ð	time shifts from 31 s to 32
	Overall Inters	ootion	0.98	36.5	D		1.01	40.7	Ð		1.00	41.7	Ð	s; NB/SB green time
		ection	<u>0.93</u>	<u>29.7</u>	<u>C</u>	-	0.94	31.1	<u>C</u>	-	1.00	41.7		shifts from 69 s to 68 s].
8. 36TH AVENUE & 31S									-					
36th Avenue	EB	LTR	0.68	32.0	С	LTR	0.70	32.7	С	-	-	-	-	
04 + 0+ - +	WB	LTR	0.68	30.3	С	LTR	0.70	31.0	С	-	-	-	-	
31st Street	NB SB	LTR LTR	0.63	17.5	B	LTR	0.66	18.5	B	-	-	-	-	-Mitigation not required.
	Overall Inters		0.65	17.6 22.5	В С	LTR	0.65 0.68	17.6 23.1	C	-	-	-	-	
9. 41ST AVENUE & VER			0.00	22.3	U	-	0.00	23.1	U	-	-	-	-	
41st Avenue	WB	LR	0.26	16.0	В	LR	0.27	16.1	В	LR	0.30	17.8	В	-Modify signal timing:
Vernon Boulevard	NB	TR	0.65	13.1	B	TR	0.69	14.0	B	TR	0.65	12.0	B	shift 1.8_2 s green time
	SB	LT	1.06	46.1	D	LT	1.09	57.7	E*	LT	1.03	34.9	C	from WB phase to NB/SB
														phase [NB/SB green time
					-								-	shifts from 31.8 32 s to
	Overall Inters	ection	0.75	31.7	С	-	0.77	38.2	D	-	0.77	25.1	С	33.6 34 s; WB green time shifts from 19.8 20 s to
														18 s].
10. 30TH AVENUE & 21	ST STREET			1				I				I		
30th Avenue	EB	LTR	0.47	37.8	D	LTR	0.47	37.8	D	-	-	-	-	
	WB	LTR	0.72	46.2	D	LTR	0.72	46.4	D	-	-	-	-	
21st Street	NB	LTR	0.51	14.5	В	LTR	<u>0.52</u>	14.6	В	_	-	_	-	
		LIN	<u>0.44</u>	<u>13.4</u>	0	LIN	<u>0.45</u>	<u>13.6</u>	6	-		-		-Mitigation not required.
	SB	LTR	1.00	30.9	С	LTR	1.01	34.4	С	-	-	-	-	
		L	0.91	22.3		<u> </u>	0.93	22.9	<u> </u>					4
	ection	0.90 0.85	28.3 23.0	С	-	0.91 0.86	30.3 23.4	С	-	-	-	-		
	1		0.00	23.0	1		<u>v.ou</u>	<u> 2.7.4</u>	1				1	

Table 22-2b (cont'd) 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No A	Action			With	Action			With M	itigation		<i>cu intersections)</i>
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
11. BROADWAY & VER		RD / 11	TH ST		AM Pe	ak Hou	r (cont	inued)						
				28.2	_	1.70	0.04	28.2	0					
Park Entrance	EB	LTR	0.01	<u>28.0</u>	С	LTR	0.01	<u>28.0</u>	С	-	-	-	-	
Broadway	WB	LTR	1.04 0.82	62.8 57.9	Е	LTR	1.06 0.84	71.8 60.1	E≛	-	-	-	-	
Vernon Boulevard	NB	LT	0.25	7.9	А	LT	0.25	7.9	A		_			
vernon Boulevard	IND	LI	0.25	2.2	A		0.25	2.2	A	-	-	-	-	
		R	0.04	6.4 1.1	А	R	0.05	6.4 1.1	А	-	-	-	-	<u>-Mitigation not required.</u> Unmitigatable Impacts
	SB	LTR	1.02	58.3	E	LTR	1.04	65.0	E*	_	_		-	enningatable impacto
	36	LIK	<u>0.93</u>	52.2	D	LIK	0.95	<u>56.4</u>	L		-	-	_	
11th Street	NB	LTR	0.38 0.37	41.2 42.1	D	LTR	0.38 0.37	41.2 42.1	D	-	-	-	-	
	Overall Interse	oction	1.01	47.6	D		1.03	52.9	D	_	-		-	
			<u> </u>	<u>40.4</u>		-		<u>42.7</u>	U	-	-	-	-	
12. ASTORIA BOULEVA		NUE/N	0.84	WN AVE 61.6	1	2151	0.84	61.6	<u> </u>	<u> </u>	0.78	4 8.7	Đ	-Modify signal timing:
Astoria Boulevard	EB	L	0.78	56.4	E	L	0.78	<u>56.4</u>	E	L	0.81	60.1	Ē	shift 1 s green time from
		TR	0.86	54.6	D	TR	0.87	55. 4	E	TR	0.55	34.6	ę	the EB phase to the
			<u>0.82</u>	<u>52.4</u>			<u>0.83</u>	<u>53.0</u>	<u>D</u>		<u>0.87</u> 0.99	<u>56.2</u> 59.8	E	NB/SB phase [EB phase green time shifts from 25
	WB	L	0.98	63.8	E	L	0.98	63.8	E	L	0.98	<u>63.8</u>	Е	s to 24 s; NB/SB green
		TR	0.86	4 6.8	D	TR	0.86	47.0	D	TR	0.65	35.7	Đ	time shifts from 50 s to 51 s; WB green time
			0.84 0.86	<u>45.9</u> 39.2			0.84 0.89	<u>46.1</u> 42.1			<u>0.84</u> 0.74	<u>46.1</u> 29.2	<u>D</u> C	remains the same].
21st Street	NB	LTR	0.82	36.7	D	LTR	0.86	38.8	D	LTR	0.83	36.2	D	-Modify signal phasing: Add a
	SB	LTR	1.08	72.0	Е	LTR	1.10	81.6	F*	LTR	0.99	35.7	Đ	new lag phase for the EB/WB exclusive left turns.
	-			<u>70.9</u>				<u>80.4</u>			<u>1.08</u>	<u>70.4</u>	E	The existing signal phasing WB has 30 s green time; EB
	Overall Interse	ection	1.00 <u>0.99</u>	58.7 57.2	E	-	1.01 <u>1.00</u>	63.0 <u>61.4</u>	E	-	0.99 <u>1.00</u>	37.7 <u>57.7</u>	D E	has 25 s green time; NB/SB has 50 s green time; NB/SB has 50 s green time; would be modified to have the following: EB/VB will have 39 s green time; EB/VB exclusive left-tum phase will have 10 s green time; NB/SB will have 56 s green time [each phase will have 3 s amber and 2 s all red].
13. HOYT AVENUE NOP		EET		1	_					1		1	1	ſ
Hoyt Avenue North	EB	L R	0.02	40.4 47.5	D	R	0.02	40.4 47.5	D	-	-	-	-	•
	WB	L	0.90	44.1	D	L	0.92	45.8	D	-	-	-	-	1
		TR	0.25	14.8	В	TR	0.25	14.8	В	-	-	-	-	-Mitigation not required.
21st Street	NB	L	0.30	31.5 85.7	C F	L	0.30	31.7 85.7	C F	-	-	-	-	
	SB	TR	1.04	85.7 53.9	F D	TR	1.04	85.7 55.8	E	-	-	-	-	1
	Overall Interse		0.85	53.1	D	-	0.86	54.2	D		-	-	-	1
14. HOYT AVENUE SOL		EET												
Hoyt Avenue South	EB	L	0.13	30.0	С	L	0.13	30.0	С	LTR	0.61	36.2	D	-Restripe EB approach of
21st Street	NB	TR LTR	1.06 0.55	75.0 15.1	E B	TR LTR	1.06 0.55	75.0 15.2	E B	- LTR	- 0.54	- 14.6	- B	Hoyt Avenue South from one 11-ft exclusive left-turn
	SB	LTR	1.03	46.1	D		1.05	52.3	D*	LTR	1.03	45.5	D	lane and one 11-ft shared
	Overall Interse	ection	1.04	42.3	D	-	1.05	45.7	D	-	0.89	35.5	D	through-right lane to two 11- ft shared left-through-right lanes for 250 ft. -Modify signal timing: shift 1 s green time from EB phase to NB/SB phase [EB green time shifts from 37 s to 36 s; NB/SB green time shifts from 73 s to 74 s].

Table 22-2b (cont'd) 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		No Action					With Action				With M	itigation		, í
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
							eak Ho	ur						
5. ROOSEVELT ISLAND						EVARD		40.4			0.04	47.0		
Roosevelt Island Bridge	EB	TR	0.22	12.4 14.3	B	TR	0.28	13.1	B	L TR	0.34 0.64	17.0 22.0	B C	-Mitigation not required.
36th Avenue	WB	LTR	0.41	14.3	B	LTR	0.53	16.3 15.1	B		0.64 0.57	22.0 21.3	C	-Modify signal timing:
			0.33	26.6	Б С		1.06	62.9	E*					shift 4 s green time from
Vernon Boulevard	NB	LTR	0.78	19.7	B	LTR	0.93	30.3	C	LTR	0.88	22.7	c	EB/WB phase to NB/SB phase [EB/WB green
	SB	LTR	0.68	19.0	B	LTR	0.72	20.3	С	LTR	0.62	14.6	B	time shifts from 25 s to 21
	Overall Inters		0.65	19.4	в		0.80	31.1	С		0.78	19.8	B	s; NB/SB green time
	Overall inters	ection	0.59	17.2	Ь	-	<u>0.73</u>	<u>21.2</u>	C	-	0.76	19.0	Ð	shifts from 25 s to 29 s].
6. 36TH AVENUE & 21S					-		-							-
36th Avenue	EB	LTR	0.78	46.5	D	LTR	0.97	71.5	E*	LTR	0.84	47.9	D	-Modify signal timing:
	WB	LTR	0.86	50.5	D	LTR	0.89	53.7	D	LTR	0.78	42.3	D	shift 4 s green time from
21st Street	NB	LTR	0.67	17.3	В	LTR	0.79	21.5	С	LTR	0.85	27.0	С	NB/SB phase to EB/WB phase [EB/WB green
	SB	LTR	0.61	16.1	В	LTR	0.62	16.5	В	LTR	0.66	19.5	В	time shifts from 37 s to 41
	Overall Inters	ection	0.73	24.2	с	-	0.85	29.7	с	-	0.85	28.8	с	s; NB/SB green time
			0.10	-1.2	Ũ		0.00	20.1	Ũ		0.00	20.0	Ŭ	shifts from 73 s to 69 s].
7. BROADWAY & 21ST	STREET													
Broadway	EB	LTR	0.95	64.9	E	LTR	0.98	71.5	E*	LTR	0.95	63.7	E	
Dioddwdy	LD	LIIX	<u>0.83</u>	<u>51.4</u>	D	LIIX	<u>0.86</u>	<u>53.1</u>	D	LIIX	<u>0.83</u>	<u>50.4</u>	D	
	WB	LTR	1.01	77.1	Е	LTR	1.02	80.7	F* E*	LTR	0.97	66.9	Е	
			0.99	<u>71.9</u>			<u>1.00</u>	76.1 22.7			0.95	<u>63.8</u>		-Modify signal timing:
21st Street	NB	LTR	0.78 0.76	22.2 21.5	С	LTR	0.80 0.77	21.9	С	LTR	0.81 0.79	23.7 22.9	С	shift 1 s green time from
			0.70	21.2	c		0.78	22.5			0.79	23.5		NB/SB phase to EB/WB phase [EB/WB green
	SB	LTR	0.69	19.8	В	LTR	0.70	20.4	С	LTR	0.72	21.2	С	time shifts from 31 s to 32
	0		0.85	32.7	С	-	0.87	34.5	•		0.86	32.9	с	s; NB/SB green time
	Overall Interse	ection	0.83	29.7	ن	-	0.85	30.7	С	-	0.84	29.8	ι L	shifts from 69 s to 68 s].
8. 36TH AVENUE & 31S	T STREET													
36th Avenue	EB	LTR	0.81	35.8	D	LTR	0.82	36.8	D	-	-	-	-	
	WB	LTR	0.74	33.2	С	LTR	0.76	34.1	С	-	-	-	-	
31st Street	NB	LTR	0.57	16.2	В	LTR	0.57	16.2	В	-	-	-	-	-Mitigation not required.
	SB	LTR	0.48	14.4	В	LTR	0.48	14.4	B	-	-	-	-	
	Overall Inters		0.66	23.9	С	-	0.67	24.4	С	-	-	-	-	
9. 41ST AVENUE & VER	WB		0.40	15.0	В		0.18	15.0	В		0.00	16.7	Р	Mitigation not require -!
41st Avenue Vernon Boulevard	NB	LR TR	0.18	15.2 13.6	B	LR TR	0.18	15.2 14.4	B	LR TR	0.20	16.7 12.3	B	-Mitigation not required. -Modify signal timing:
	SB	LT	0.66	13.6	B	LT	0.70	14.4	B	LT	0.66	12.3	B	shift 1.8-2 s green time
	30		0.04	13.2	D		0.00	14.0			0.04	12.1	D	from WB phase to NB/SB
														phase [NB/SB green time
														shifts from 31.8 32 s to
														33.6 34 s; WB green time
	Overall Inters	ection	0.48	13.6	в	-	0.50	14.3	в	-	0.50	12.5	в	shifts from 19.8 <u>20</u> s to
														18 s]. [Measures reflect
														improvements needed for
														the weekday AM and PM
														peak periods.]
10. 30TH AVENUE & 21	ST STREET													
30th Avenue	EB	LTR	0.33	34.4	С	LTR	0.33	34.4	С	-	-	-	-	
	WB	LTR	0.50	38.9	D	LTR	0.50	38.9	D	-	-	-	-	
21st Street	NB	LTR	0.72	18.6	В	LTR	0.73	19.1	В	-	-	-	-	-Mitigation not required.
	SB	LTR	0.78	20.0	В	LTR	0.80	20.7	С	-	-	-	-	4
	Overall Interse	ection	0.68	21.5	С	-	0.70	21.9	С	-	-	-	-	

Table 22-2b (cont'd) 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		No Action		With Action						itigation		a mersections)		
			<u> 110 /</u>	Control	1		<u></u>	Control				Control	i –	
Intersection	Approach	M∨t.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mitigation Measure
				М	idday F	Peak Ho	our (co	ntinued)						
I. BROADWAY & VERN	ION BOULEVA	RD / 11	TH ST											
ark Entrance	EB	LTR	0.02	26.2	С	LTR	0.02	26.2	С	-	-	-	-	
			0.89	<u>27.0</u> 47.0	-		0.91	<u>27.0</u> 49.2	-					
roadway	WB	LTR	0.82	53.8	D	LTR	0.84	49.2 55.0	D	-	-	-	-	
na an Daviday and	ND			8.3	•			8.4						
ernon Boulevard	NB	LT	0.26	2.7	A	LT	0.27	<u>2.8</u>	A	-	-	-	-	
		R	0.17	7.6	А	R	0.18	7.6	А	-	-	-	-	-Mitigation not required.
			0.56	<u>2.8</u> 27.2			0.57	<u>2.9</u> 27.7						
	SB	LTR	0.55	31.5	С	LTR	0.65	32.3	С	-	-	-	-	
	ND		0.22	32.9		1.70		32.9	с					
1th Street	NB	LTR	<u>0.21</u>	34.0	С	LTR	0.22	33.6	C	-	-	-	-	
	Overall Interse	ection	0.72	25.6	с	-	0.74	26.3	с	-	-	-	-	
2. ASTORIA BOULEVA				26.2		2467		<u>26.6</u>	-					
2. ASTORIA DOULEVAR						2131 3	SIREE				0.26	24.0	C	-Modify signal timing:
storia Boulevard	EB	L	0.26	34.9	С	L	0.26	34.9	С	L	0.20	35.8	Ď	shift 1 s green time from
		TD	0.40	36.3	D	TD	0.41	36.5	D	TR	0.36	32.0	C	the EB phase to the
		TR	<u>0.39</u>	<u>36.2</u>	D	TR	<u>0.40</u>	<u>36.3</u>	D	IR	<u>0.41</u>	<u>37.3</u>	D	NB/SB phase [EB phase
	WB	L	0.86	53.0	D	L	0.86	53.0	D	L	0.87	4 7.3	D	green time shifts from 34 s to 33 s; NB/SB green
				36.2				36.3			0.86 0.38	<u>53.0</u> 31.9	c	time shifts from 37 s to 38
		TR	0.43	<u>36.2</u> 36.1	D	TR	0.44	36.2	D	TR	0.30	36.2	D	s; WB green time
1 at Streat	ND		1 1 2		F		4 4 7		F*		0.67	25.2	ç	remains the same].
1st Street	NB	LTR	1.13	102.1	F	LTR	1.17	121.8	F	LTR	1.13	<u>100.2</u>	E	-Modify signal phasing: Add a new lag phase for the
	SB	LTR	1.00	56.1	Е	LTR	1.04	65.9	E*	LTR	0.68	25.5	ç	EB/WB exclusive left turns.
											<u>1.01</u>	<u>56.6</u>	<u>E</u>	The existing signal phasing
	Overall Interse	ction	0.81 <u>0.80</u>	63.9 <u>64.0</u>	E		0.82	73.0 <u>73.1</u>	E	-	0.79 <u>0.82</u>	29. 4 <u>63.8</u>	С Е	has 34 s green time; NB/SB has 37 s green time] would be modified to have the following: EB/WB will have 39 s green time; EB/WB exclusive left tum phase will have 10 s green time; NB/SB will have 56 s green time [each phase will have 3 s amber and 2 s all red].
B. HOYT AVENUE NOR	TH & 21ST STR	REET												
oyt Avenue North	EB	L	0.11	42.0	D	L	0.11	42.0	D	-	-	-	-	
	14/5	R	0.13	42.5	D	R	0.13	42.5	D	-	-	-	-	
	WB	L TR	0.69	38.5 14.2	D B	L TR	0.72	39.3 14.2	D B	-	-	-	-	
1st Street	NB	L	0.17	25.2	В С	L	0.17	25.2	В С	-	-	-	-	Mitigation not required
		T	0.77	43.0	D	T	0.77	43.1	D	-	-	-	-	
	SB	TR	0.57	33.4	C	TR	0.58	33.5	C	-	-	-	-	
	Overall Interse	ction	0.61	36.6	D	-	0.62	37.1	D	-	-	-	-	
4. HOYT AVENUE SOUT		EET												
oyt Avenue South	EB	L	0.21	31.6	С	L	0.21	31.6	С	LTR	0.32	32.8	С	-Mitigation not required.
	ND	TR	0.41	35.5	D	TR	0.41	35.5	D	-	-	-	-	 Restripe EB approach of Hovt Avenue South from
1st Street	NB SB	LTR LTR	0.43	13.3 15.9	B	LTR LTR	0.44 0.62	13.4 16.2	B	LTR LTR	0.44	13.4 16.2	B	one 11-ft exclusive left-
	Overall Interse		0.54	17.8	В	-	0.55	17.9	В	-	0.62	17.7	в	turn lane and one 11-ft shared through-right lane to two 11-ft shared left- through-right lanes for 250 ft. [Measures reflect improvements needed for the weekday AM peak
	Overall Interse	ction	0.54	17.8	В	-	0.55	17.9	В	-	0.52	17.7	В] impre

Table 22-2b (cont'd) 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		Action			With Action					itigation				
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
							k Hour	r						
5. ROOSEVELT ISLAND	BRIDGE / 36TH		UE & V	ERNON	BOUL	EVARD		r	r	r			1	[
Roosevelt Island Bridge	EB	L	0.46	14.3	В	L	0.51	15.1	В	L	0.60 0.58	19.5 <u>18.3</u>	В	
		TR	0.59	15.6	В	TR	0.78	19.8	В	TR	0.92 0.88	33.2 27.9	С	-Modify signal timing:
36th Avenue	WB	LTR	0.28	12.8	В	LTR	0.34	13.6	в	LTR	0.50 0.44	20.3 18.0	С В	shift 4 <u>3</u> s green time from EB/WB phase to
Vernon Boulevard	NB	LTR	1.15 1.15	88.6 86.8	F E	LTR	1.39 1.20+	194.5 <u>192.0</u>	F*	LTR	1.10 1.16	63.5 88.5	₽ E	NB/SB phase [EB/WB green time shifts from 25
	SB	LTR	0.85	25.3	С	LTR	0.87	27.4	С	LTR	0.75 0.78	17.5 19.2	В	s to 21 22 s; NB/SB green time shifts from 2 s to 29 28 s].
	Overall Inters	ection	0.88	4 <u>2.8</u> 42.2	D	-	1.09	77.4 76.6	Е	-	1.03 1.04	36.4 43.4	D	s to 29 <u>26</u> sj.
6. 36TH AVENUE & 21S	T STREET												1	
36th Avenue	EB	LTR	0.51	35.1	D	LTR	0.78	42.0	D	-	-	-	-	
	WB	LTR	0.79	45.5	D	LTR	0.83	48.4	D	-	-	-	-	
21st Street	NB	LTR	0.92	24.8	С	LTR	0.92	24.8	С	-	-	-	-	-Mitigation not required.
	SB	LTR	0.69	17.8	В	LTR	0.70	18.2	В	-	-	-	-	
	Overall Inters	ection	0.87	25.5	С	-	0.89	26.7	С	-	-	-	-	
7. BROADWAY & 21ST	STREET													1
Broadway	EB	LTR	1.13	107.6 <u>106.0</u>	F	LTR	1.16	120.4 118.8	F*	LTR	1.12 1.11	102.5 <u>99.5</u>	F	-Modify signal timing:
	WB	LTR	1.17 <u>1.15</u>	125.7 <u>115.7</u>	F	LTR	1.19 <u>1.17</u>	134.4 <u>124.3</u>	F*	LTR	1.13 1.11	108.0 <u>98.7</u>	F	shift 1 s green time from NB/SB phase to EB/WB
21st Street	NB	LTR	0.91 0.90	26.7 26.4	С	LTR	0.93 0.92	28.2 27.8	С	LTR	0.94 0.94	30.1 29.6	С	phase [EB/WB green time shifts from 31 s to 32
	SB	LTR	0.72 0.70	20.6 20.1	С	LTR	0.73 0.71	20.9 20.5	С	LTR	0.74 0.72	21.9 21.4	С	s; NB/SB green time shifts from 69 s to 68 s].
	Overall Inters	ection	0.99 0.98	4 2.8 41.4	D	-	1.01 1.00	4 5.8 44.4	D	-	1.00 0.99	4 2.6 41.0	D	
8. 36TH AVENUE & 31S	T STREET													•
36th Avenue	EB	LTR	0.80	32.0	C	LTR	0.85	34.9 32.2	C	-	-	-	-	
	WB	LTR	0.71	31.6	С	LTR	0.73 0.70	<u>32.3</u> 19.2	С	-	-	-	-	
31st Street	NB SB	LTR LTR	0.69	19.0 14.5	B	LTR LTR	<u>0.69</u> 0.48	<u>19.0</u> 14.5	B	-	-	-	-	-Mitigation not required.
	Overall Inters		0.48	23.2	C	-	0.76	14.5 24.2	C	-	-	-		
9. 41ST AVENUE & VER							0.75							
41st Avenue	WB	LR	0.26	16.1	В	LR	0.32	16.9	В	LR	0.35	18.7	В	-Modify signal timing:
Vernon Boulevard	NB	TR	1.03	39.7	D	TR	1.05	47.7	D*	TR	1.00	29.9	C	shift 1.8 2 s green time
Former Dealeraid	SB	LT	0.93	24.7	C	LT	0.99	35.4	D	LT	0.88	18.5	B	from WB phase to NB/SB
	Overall Inters	ection	0.73	31.8	с	-	0.77	40.1	D	-	0.77	24.3	с	phase [NB/SB green time shifts from 31.8 <u>32</u> s to 33.6 <u>34</u> s; WB green time shifts from 19.8 20 s to
													1	18 s
10. 30TH AVENUE & 21	ST STREET		·	·						·	·			
30th Avenue	EB	LTR	0.32	34.1	С	LTR	0.33	34.2	С	-	-	-	-	
	WB	LTR	0.48	38.0	D	LTR	0.48	38.0	D	-	-	-	-	
21st Street	NB	LTR	0.81 0.76	20.6 <u>18.7</u>	С В	LTR	0.83 <u>0.78</u>	21.4 19.3	С В	-	-	-	- Mitigation not rogu	-Mitigation not required.
	SB	LTR	0.63 0.59	16.5 <u>15.6</u>	В	LTR	0.65 0.60	16.8 <u>15.8</u>	В	-	-	-	-	
	Overall Inters	ection	0.70 0.66	20.8 <u>19.5</u>	C B	-	0.71 0.68	21.2 <u>19.8</u>	С в	-	-	-	-	

Table 22-2b (cont'd) 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

	No Action				With Action					With M	itigation			
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
					PM Pe	ak Hou	r (cont	inued)						
11. BROADWAY & VEI	RNON BOULEVA	RD / 11	IHSI	1		r		22.2	1	1		1	1	
Park Entrance	EB	LTR	0.03	33.2 33.5	С	LTR	0.03	33.2 33.5	С	-	-	-	-	
Dreadway	\M/D		0.84	52.6	D		0.86	54.2	Ð					
Broadway	WB	LTR	<u>0.73</u>	<u>53.9</u>	D	LTR	<u>0.78</u>	<u>58.3</u>	E	-	-	-	-	
Vernon Boulevard	NB	LT	0.46	9.3 2.2	Α	LT	0.47	9.4 2.3	А	-	-	-	-	
				<u> 2.2</u> 6.3		_		<u>2.3</u> 6.4						
		R	0.13	2.5	A	R	0.14	2.6	A	-	-	-	-	-Mitigation not required
	SB	LTR	0.62	29.3	С	LTR	0.63	29.6	С	-	-	-	-	
			<u>0.69</u>	<u>34.4</u>			<u>0.70</u>	35.0	-					
11th Street	NB	LTR	0.33	38.3 <u>39.5</u>	D	LTR	0.33	38.3 39.5	D	-	-	-	-	
	Overall Inters	oction	0.86	24.6	с		0.87	24.9	с				-	
			=	22.6		-	=	23.6	U	-	-	-	-	
12. ASTORIA BOULEV	ARD / 27TH AVE	NUE / N	NEWTO	WN AVE	ENUE 8	21ST	STREE	Т	-	1				
Astoria Boulevard	EB	L	0.47	42.4	D	L	0.47	42.4	D	L	0.41 0.49	28.0 43.6	С D	<u>-Modify signal timing: shift</u> s green time from the EB
			0.78	4 8.9	_	-	0.80	49.6	_		0.57	<u>35.1</u>		phase to the NB/SB phase
		TR	0.76	<u>48.0</u>	D	TR	0.78	48.6	D	TR	0.81	50.7	D	[EB phase green time shifts
	WB	L	0.89	64.8	Е	L	0.89	64.8	Е	L	0.76	4 3.9	Ð	from 28 s to 27 s; NB/SB green time shifts from 53 s
			0.88 0.78	<u>63.5</u> 51.5			0.88 0.78	<u>63.5</u> 51.7			0.88 0.45	<u>63.5</u> 32.9	E C	to 54 s; WB green time
		TR	0.76	51.0	D	TR	0.77	51.2	D	TR	0.43	51.2	D	remains the same].
21st Street	NB	LTR	1.04	54.2	D	LTR	1.08	69.6	E*	LTR	0.99	38.3	D	-Modify signal phasing: Add
	ND	LIK	1.02	<u>48.5</u>			<u>1.06</u>	<u>62.9</u>	-	LIIX	<u>1.03</u>	<u>50.6</u>		EBAVB exclusive left turns.
	SB	LTR	0.90	36.3	D	LTR	0.93	38.1	D	LTR	0.88 0.91	32.7 36.0	C D	The existing signal phasing
		1									0.01	00.0		[WB has 24 s green time; EB has 28 s green time; NB/SB
	Overall Interse	ection	0.94 <u>0.92</u>	4 8.0 <u>45.9</u>	D		0.96 <u>0.94</u>	53.6 <u>51.1</u>	D	-	0.93 <u>0.94</u>	35.5 <u>46.8</u>	D	has 53 s green time] would be modified to have the following: EBAWB will have 39 s green time; EBAWB exclusive left tum phase will have 10 s green time; NB/SI will have 56 s green time [each phase will have 3 s amber and 2 s all red].
13. HOYT AVENUE NO	RTH & 21ST STR	REET												
Hoyt Avenue North	EB	L	0.09	41.8	D	L	0.09	41.8	D	-	-	-	-	
		R	0.17	43.1	D	R	0.17	43.1	D	-	-	-	-	
	WB	TR	0.61 0.29	36.8 15.7	B	TR	0.63	37.3 15.7	B	-	-	•	-	1
21st Street	NB	L	0.29	26.1	C	L	0.29	26.1	C	-	-	-	-	-Mitigation not required.
		T	1.09	90.0	F	T	1.09	92.4	F	-	-	-	-	1
	SB	TR	0.76	39.0	D	TR	0.76	39.0	D	-	-	-	-	
	Overall Interse		0.73	52.9	D	-	0.74	53.8	D	-	-	-	-	
14. HOYT AVENUE SO		1 .	0.17	00.0	<u> </u>		0.17	00.0		1 70	0.17	045		Niliantian at a second
Hoyt Avenue South	EB	L TR	0.17 0.75	30.8 44.3	C D	L TR	0.17 0.75	30.8 44.3	C D	LTR	0.47	34.5	C	-Mitigation not required. -Restripe EB approach of Ho
								44.3 29.5				- 29.5	<u> </u>	Avenue South from one 11-f
21st Street	NB	LTR	0.92	26.3	С	LTR	0.94	28.1	С	LTR	0.94	28.1	С	exclusive left-turn lane and one 11-ft shared through-righ
	SB	LTR	0.89	28.0	С	LTR	0.91	29.3	С	LTR	0.91	29.3		lane to two 11-ft shared through-rigr
	00		0.03	<u>27.4</u>			0.01	23.5			0.01	23.5	С	through-right lanes for 250 ft
	Overall Interse	ection	0.86	29.4 29.2	с	-	0.87	31.3 30.7	с	-	0.78	30.2 29.6	с	[Measures reflect improvements needed for the weekday AM peak period.]

Control delay is measured in seconds per vehicle.
 Overall intersection V/C ratio is the critical lane groups' V/C ratio.
 Denotes a significant impact.

Table 22-3a 2038 No Action, With Action, and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

		No Action					With	Action			With	Vitigatio	n	
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay		Mvt. Peak	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mitigation Measure
1. EAST/WEST ROAD & MAIN	STREET				AIV	ГРеак	Hour							
West Road	EB	LT	-	7.2	А	LT	-	8.0	А	LT	-	8.2	А	-Mitigation not required.
Main Street	SB	LR	-	7.4	A	LR	-	11.3	B	LR	-	13.9	B	-Conditions shown reflect
														additional U-turns that
												13.9		would use this intersection
	Overall Inte	ersection	-	7.3	Α	-	-	11.2	в	-	-	13.9	в	because of the proposed
				_										elimination of the traffic triangle at West Road and
														Main Street.
2. WEST ROAD & MAIN STRE	ET													
West Road	EB	LR	-	9.3	Α	LR	-	12.7	В	LR	0.36	15.8	В	-Mitigation not required.
West Road (south of island)	EB	LR	-	11.5	В	LR	-	16.4	С	-	-	-	-	-Install traffic signal with
Main Street	NB	LT	-	10.1	В	LT	-	12.1	В	Т	0.28	10.7	В	the following timing plan:
	SB	TR	-	9.6	Α	TR	-	25.2	D	Т	0.69	17.3	В	EB will have 22 s green time: NB/SB will have 28 s
														green time [each phase will
														have 3 s amber and 2 s all
														red time].
														-Reconfigure to eliminate
	Overall Int				_				_				_	traffic triangle and
	Overall Inte	ersection	-	10.1	в	-	-	19.4	С	-	0.55	15.6	в	consolidate turning movements at one
														intersection.
														[Measures reflect
														improvements needed for
														the weekday PM peak
3. ROOSEVELT ISLAND BRID		MAINCT	DEET											period.]
Roosevelt Island Bridge Ramp	WB	LR	REEI	16.2	С	LR	-	110.6	F*	LR	0.91	30.9	С	-Install traffic signal with
Rooseven Island Bridge Ramp	NB	T	-	10.2	В	T	-	11.6	В	T	0.12	13.1	В	the following timing plan:
Main Street		R	-	11.4	В	R	-	17.9	C	R	0.69	24.1	C	WB will have 28 s green
	SB	LT	-	12.8	В	LT	-	15.9	C	LT	0.47	17.8	B	time; NB/SB will have 22 s
														green time [each phase will
	Overall Inte	ersection	-	13.9	в	-	-	67.9	F	-	0.81	26.4	С	have 3 s amber and 2 s all red time].
4. ROOSEVELT ISLAND BRID	GE & MOTO	RGATE G	ARAG		ANCE	/ EXIT								icu tinej.
Roosevelt Island Bridge	EB	LT	-	8.5	A	LT	-	9.6	А	-	-	-	-	
Motorgate Garage Exit	NB	LR	-	11.5	В	LR	-	13.0	В	-	-	-	-	-Mitigation not required.
	Overall Inte	ersection	1	1.5	Α	-	-	1.4	Α	-	-	-	-	
					Midd	ay Pea	ak Hou	ır						
1. EAST/WEST ROAD & MAIN	1				1	r			-	1	r		r	
West Road	EB	LT	-	7.6	A	LT	-	8.1	A	LT	-	8.2	A	-Mitigation not required.
Main Street	SB	LR	-	7.4	A	LR	-	9.1	A	LR	-	9.7	A	-Conditions shown reflect additional U-turns that
														would use this intersection
	0											0.5		because of the proposed
	Overall Inte	ersection	-	7.5	Α	-	-	9.0	Α	-	-	9.5	A	elimination of the traffic
					1				1					triangle at West Road and
	l				I		L		I					Main Street.

Table 22-3a (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

			No Ao	tion			With	Action			<u>`</u>	Mitigatio		zeu mierseetions)
			AL	Control	1		<u></u>	Control				Control	† – –	1
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
				Midd	ay Pea	ak Hou	r (con	tinued)						
2. WEST ROAD & MAIN STRE	ET													
West Road	EB	LR	-	8.5	Α	LR	-	12.6	В	LR	0.48	17.7	В	-Mitigation not required.
West Road (south of island)	EB	LR	-	10.9	В	LR	-	13.4	В	-	-	-	-	-Install traffic signal with
Main Street	NB	LT	-	9.4	Α	LT	-	11.4	В	Т	0.22	10.1	В	the following timing plan:
Main Street	SB	TR	-	8.7	Α	TR	-	14.5	В	Т	0.47	12.8	В	EB will have 22 s green
	Overall Inte		-	9.4	A	-	-	13.1	в	-	0.47	13.7	в	time; NB/SB will have 28 s green time [each phase will have 3 s amber and 2 s all red time]. -Reconfigure to eliminate traffic triangle and consolidate turning movements at one intersection. [Measures reflect improvements needed for the weekday PM peak period.]
3. ROOSEVELT ISLAND BRID	GE RAMP &	MAIN ST	REET											
Roosevelt Island Bridge Ramp	WB	LR	-	10.4	В	LR	-	21.5	С	LR	0.54	14.1	В	-Mitigation not required.
	NB	Т	-	9.3	Α	Т	-	10.5	В	Т	0.14	13.2	В	-Install traffic signal with
Main Street		R	-	9.2	Α	R	-	16.9	С	R	0.82	32.6	С	the following timing plan:
	SB	LT	-	10.8	В	LT	-	13.4	В	LT	0.47	18.2	В	WB will have 28 s green time; NB/SB will have 22 s
4. ROOSEVELT ISLAND BRID	Overall Inte		-	10.0	В	-	-	17.6	с	-	0.67	21.2	с	green time [each phase will have 3 s amber and 2 s all red time]. [Measures reflect improvements needed for the weekday AM and PM peak periods.]
Roosevelt Island Bridge		LT	-	7.7		LT	- 1	8.2	А	1	-		-	
Motorgate Garage Exit	NB		-	10.1	B		-	11.4	B	-	-	-	-	-Mitigation not required.
	Overall Inte		-	0.9	A	-	-	0.7	A	-	-	-	-	-miligation not required.
	o voi un inte			0.0		Peak		0.1						
1. EAST/WEST ROAD & MAIN	STREET				1.14	I Cuk	noui							
West Road	EB	LT	-	7.4	Α	LT	-	7.8	Α	LT	-	7.9	Α	-Mitigation not required.
Main Street	SB	LR	-	7.3	A	LR	-	8.5	A	LR	-	9.0	A	-Conditions shown reflect
	Overall Inte		-	7.3	A	-	-	8.4	A	-	-	8.8	A	additional U-turns that would use this intersection because of the proposed elimination of the traffic triangle at West Road and Main Street.
2. WEST ROAD & MAIN STRE			1											
West Road	EB	LR	-	9.0	Α	LR	-	84.8	F*	LR	0.96	42.4	D	-Install traffic signal with
West Road (south of island)	EB	LR	-	10.9	В	LR	-	12.2	В	-	-	-	-	the following timing plan:
Main Street	NB	LT	-	10.4	В	LT	-	17.3	С	Т	0.22	11.9	В	EB will have 25 s green time; NB/SB will have 25 s
	SB Overall Inte	TR	-	9.0 9.9	A A	<u>-</u>	-	16.8 48.6	E	-	0.36 0.66	<u>13.4</u> 30.6	С	green time [each phase will have 3 s amber and 2 s all red time]. -Reconfigure to eliminate traffic triangle and consolidate turning movements at one intersection.

Table 22-3a (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

			No A	ction			With	Action			With I	Mitigatio	<u>1</u>	
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
				PM	Peak	Hour ((conti	nued)						
3. ROOSEVELT ISLAND BRID	GE RAMP &	MAIN ST	REET											
Roosevelt Island Bridge Ramp	WB	LR	-	11.5	В	LR	-	20.2	С	LR	0.58	19.1	В	-Install traffic signal with
	NB	Т	-	9.7	Α	Т	-	10.7	В	Т	0.08	9.1	Α	the following timing plan:
Main Street		R	-	10.0	Α	R	-	37.6	E*	R	0.86	28.6	С	WB will have 22 s green
	SB	LT	-	15.4	С	LT	-	23.5	С	LT	0.63	16.9	В	time; NB/SB will have 28 s
	Overall Inte	ersection	-	12.7	в	-	-	28.0	D	-	0.74	22.0	с	green time [each phase will have 3 s amber and 2 s all red time].
4. ROOSEVELT ISLAND BRID	GE & MOTO	RGATE G	ARAC	JE ENTR	ANCE	/ EXIT	•							
Roosevelt Island Bridge	EB	LT	-	7.9	Α	LT	-	8.3	Α					
Motorgate Garage Exit	NB	LR	-	13.0	В	LR	-	20.0	С					-Mitigation not required.
	Overall Inte	ersection	-	1.0	Α	-	-	1.1	Α					
Notes: (1) Control delay is measured ir (2) Overall intersection V/C ration Denotes a significant impact.			ups' V/	'C ratio.										

Table 22-3b 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action		1	With M	itigation		
Intersection	Approach	M∨t.	v/c	Control Delay	LOS	Mvt.	V/C	Control Delay	LOS	Mvt.	V/C	Control Delay		Mitigation Measure
						AM	Peak H	our						
5. ROOSEVELT ISLA	ND BRIDGE / 3	6TH A\	/ENUE	& VERNO	N BOU	LEVAF	۶D							
Roosevelt Island Bridge	EB	L	0.31	13.3	В	L	0.46	16.3	В	Ŀ	<u>0.55</u>	<u>27.5</u>	<u>C</u>	
		TR	0.62	17.7	В	TR	0.77	22.5	С	<u>TR</u>	<u>0.83</u>	<u>35.3</u>	<u>D</u>	 Partially Mitigated Modify the cycle length
36th Avenue	WB	LTR	1.20+ 0.43	214.4 <u>14.5</u>	ĻВ	LTR	0.64	18.8	В	<u>LTR</u>	<u>0.78</u>	<u>35.1</u>		from 60 s to 90 s. EB/WB areen time is 35 s: NB/SB
Vernon Boulevard	NB	LTR	1.20+ <u>1.16</u>	336.7 <u>91.6</u>	F	LTR	1.20+	556.3 <u>358.8</u>	F*	<u>LTR</u>	<u>1.20+</u>	<u>204.5</u>	Ē	green time is 45 s; each phase has 3 s of amber ar
	SB	LTR	<u>1.14</u>	<u>87.6</u>	Ē	LTR	1.20+	385.7 <u>133.7</u>	F*	<u>LTR</u>	<u>1.04</u>	<u>52.5</u>	D	2 s of red time. Unmitigatable Impacts
	Overall Inters	ection	1.16 0.89	199.1 61.5	F	-	1.20+	287.0 144.9	F		<u>1.15</u>	<u>84.3</u>	E	

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

Intersection Approach Mrt. VC Control Delay LOS Mrt. VC Delay LO				No	Action			With	Action			With M	itigation		<u>Izeu Intersections</u>
Intersection Myt. V/C Delay LOS Myt. V/C Delay LOS Mitigation Measure A series streter Soft Avanue EB U/R 0.3 Main and the surger Main and the surger Mitigation Measure Soft Avanue EB U/R 0.4 T 0.2 1 1 0 <th< th=""><th></th><th></th><th></th><th><u></u></th><th></th><th></th><th></th><th></th><th></th><th></th><th>-</th><th></th><th></th><th></th><th></th></th<>				<u></u>							-				
6. 36TH AVENUE & 215T STREET 1.TR 0.9 E TR 1.20 1.10 1.11 <th1.11< th=""><th>Intersection</th><th>Approach</th><th>Mvt.</th><th>V/C</th><th></th><th>LOS</th><th>Mvt.</th><th>V/C</th><th></th><th>LOS</th><th>M∨t.</th><th>V/C</th><th></th><th>LOS</th><th>Mitigation Measure</th></th1.11<>	Intersection	Approach	Mvt.	V/C		LOS	Mvt.	V/C		LOS	M∨t.	V/C		LOS	Mitigation Measure
Bith Avenue EB LTR 0.91 F* L 0.835 54.3 D WB LTR 1.02 68.4 E LTR 1.17 123.1 F* L 0.45 54.3 D WB LTR 1.02 68.4 E 1.7 1.7 1.73 1.7 1.03 7.2.6 E reval lane to 1.14 exal ante 1.14						AM	Peak	Hour (c	ontinued))					·
Description Description Performant restripe EB Perfo	6. 36TH AVENUE &	21ST STREET													
WB LTR 1.02 68.4 E LTR 1.02 J.2 L 0.47 D approach from one 25-ft 21st Street NB LTR 0.40 13.0 B LTR 0.41 13.2 B LTR 0.38 10.8 B 21st Street NB LTR 1.14 88.8 F LTR 1.20 115.1 FP LTR 1.14 85.0 F W1Arking for 200 ft. 88.8 F LTR 1.20 115.1 FP LTR 1.14 85.0 F with parking for 200 ft. 4 68.5 E - 1.20+ 104.5 F - 1.10 66.9 E - 1.20+ 104.5 F - 1.10 66.9 E - 1.20+ 104.5 F - 1.10 66.9 E - 1.20+ 120.5 F - 1.10 66.9 E - - - <td< td=""><td>36th Avenue</td><td>EB</td><td>LTR</td><td>0.91</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>	36th Avenue	EB	LTR	0.91											
NB LTR LTR <thltr< th=""> <thltr< th=""> <thltr< th=""></thltr<></thltr<></thltr<>															
21 at Street NB LTR 0.40 13.0 B LTR 0.41 13.2 B LTR 0.26 Los B LTR 0.26 LS B LTR 1.14 86.0 F LTR 1.10 66.9 E Set of through-ight lane with set of 1.14 Column and restripe WS approach for 200 ft. Set of through-ight lane with set of 1.14 Column and restripe WS approach for 200 ft. Set of through-ight lane with set of 1.14 Column and restripe WS approach for 200 ft. Set of through-ight lane with set of 1.14 Column and the set of 1.14 Column and restripe WS appr		WB													
2.10 stream 0.05 LTR 0.04 102 0 LTR 1.14 88.8 F LTR 1.20 1.14 1.14 88.8 F LTR 1.20 1.14 1.14 88.8 F LTR 1.20 1.15 F LTR 1.14 88.0 F south and restripe VB and restripe VB and restripe VB approach from one 25-ft travel lane core 25-ft assouth and restripe VB approach from one 25-ft approach from one 25-ft assouth and restripe VB approach from one 25-ft approach	04 -+ 0++	ND													
Overall Intersection 1.10 69.5 E - 1.20+ 104.5 F - 1.10 66.9 E - Shift centreline (T to the south and restine VB south and	21st Street														
Broadway EB LTR 1.20+ 334.7 1.62 222.4 LTR 1.20+ 366.0 222.4 F* L 0.39 42.2 D EB approach for 200 ft from EB approach for 200 ft from WB LTR 1.20+ 14.20+ 17.1 F LTR 1.20+ 215.4 F* L 0.88 59.3 E E EB approach for 200 ft from WB LTR 1.24+ 174.1 F LTR 1.20+ 215.4 F* L 0.88 59.3 E receiving side for 250 ft from 21st Street NB LTR 0.51 16.4 B LTR 0.52 16.6 B LTR 0.50 14.7 B 21st Street NB LTR 0.51 16.4 B LTR 0.52 16.6 B LTR 0.50 14.7 B approach form one 2.24 traveliane with parking 100 con 10-ft exclusive left-turn lane and one 15-ft traveliane with parking 100 con 10-ft exclusive left-turn lane and one 15-ft traveliane with parking 100 <t< td=""><td></td><td>Overall Inters</td><td>•</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>with parking for 200 ft. -Shift centerline 6 ft to the south and restripe WB approach from one 25-ft travel lane to 11-ft exclusive left-turn lane, one 20-ft shared through-right lane with parking for 125 ft. -Modify signal timing: shift 4 s green time from EB/WB phase to NB/SB phase [EB/WB] green time shifts from 37 s to 33 s; NB/SB green time shifts from 73 s to</td></t<>		Overall Inters	•												with parking for 200 ft. -Shift centerline 6 ft to the south and restripe WB approach from one 25-ft travel lane to 11-ft exclusive left-turn lane, one 20-ft shared through-right lane with parking for 125 ft. -Modify signal timing: shift 4 s green time from EB/WB phase to NB/SB phase [EB/WB] green time shifts from 37 s to 33 s; NB/SB green time shifts from 73 s to
Bildedway EB LIR 1.20+ 202.5 E LIR 1.20+ 222.4 F = association	7. BROADWAY & 21	IST STREET								_					-
WB LTR 4.20+ 1.14 174.3 124.1 F LTR 1.20+ 165.4 215.4 F* L 0.82 66.9 E Shift centratine 3.11 to the intersection. 21st Street NB LTR 0.56 47.4 B LTR 0.65 48.4 B LTR 0.50 14.7. B 0.50 14.7. B 0.50 16.4 B LTR 0.50 14.7. B 0.50 14.7. F LTR 1.11 74.1 E 1.11 74.1 E 1.11 74.1 E 1.11 74.1 E 1.11 1.11 74.1 E 1.11 1.11 74.1 E	Broadway	EB	LTR	1.20+			LTR	1.20+		F*					EB approach for 200 ft from
WB LIR 1.14 124.1 P LIR 1.20+ 165.4 P = Max Max Shift centrating 3 fit to the north and restripe EB approach from one 22.1 21st Street NB LTR 0.65 47.4 B LTR 0.50 14.7 B north and restripe EB approach from one 22.1 21st Street NB LTR 0.45 16.4 B LTR 0.50 14.7 B north and restripe EB approach from one 22.1 SB LTR 1.46 97.9 F LTR 1.16 97.1 F* LTR 1.11 74.1 E north and restripe WB approach from one 22.1 SB LTR 1.46 97.9 F LTR 1.16 97.1 F* LTR 1.11 74.1 E Shift centrefine 3 ft tavel lane with parking 10 one 10-ft exclusive left-turn lane for 200.ft. Shift centrefine 7 ft to the south and restripe WB approach from one 22.1 ft ravel lane with parking 10 Shift centrefine 7 ft to the south and restripe WB approach from one 22.1 ft ravel lane with parking 10 Shift centrefine 15.1 Shift centrefine 15.1 Shift centrefine 15.1 Shift avel lane with parking 10 S			-			-	-	-		-	<u>TR</u>	<u>0.88</u>	<u>59.3</u>	E	the intersection and the EB receiving side for 250 ft from
And Algorithm And Algorithm<		WB	LTR			F	LTR	1.20+		F*	F	<u>0.82</u>	<u>66.9</u>	E	
21st Street NB LTR 0.65 17.4 0.51 B LTR 0.69 0.52 16.6 16.6 B LTR 0.50 14.7 16.6 B Image: Constraint of the column of the col			-	-	-	-	-	-	-	-	<u>TR</u>	<u>0.67</u>	<u>44.8</u>	D	
SB LTR 1.11 77.6 E LTR 1.16 97.1 F = Image: Line for 200 ft. =Shift centerline 7 ft to the south and restripe WB approach from one 22-ft travel lane with parking to one 10-ft exclusive left-turn lane and one 19-ft travel lane with parking to 250 ft. -Modify signal timing: shift 2 s green time to the all red time for the EB/WB phase and shift 3 s green time shifts from 31 s to 29 s; MBCSB green time shifts from 59 s to 72 s; LPI phase to the NB/SB phase [EB/WB green time shifts from 10 s to 7 s]. Unmitigatable Impacts F Image: LTR 0.79 38.1 D LTR 0.84 42.0 D Image: LTR	21st Street	NB	LTR	<u>0.51</u>	<u>16.4</u>		LTR	0.52	16.6	В		<u>0.50</u>	<u>14.7</u>		travel lane with parking to one 10-ft exclusive left-turn
Overall Intersection 1.20+ 145.7 1.18 F - 1.20+ 140.7 98.9 F = 1.04 55.9 E South and restripe WB approach from one 22-ft travel lane with parking to one 10-ft exclusive left-turn lane and one 19-ft travel lane with parking for 250 ft. -Modify signal timing: shift 3 s green time to the all red time for the EB/WB phase and shift 3 s green time to the all red time for the EB/WB phase and shift 3 s green time shifts from the LPI phase to the NB/SB phase [EB/WB green time shifts from 31 s to 29 s; NB/SB green time shifts from to s to 7 s]. Unmitigatable Impacts 8. 36TH AVENUE & 31ST STREET - LTR 0.84 42.0 D - - 8. 36TH AVENUE & 31ST STREET - LTR 0.84 42.0 D - - 31st Street NB LTR 0.73 20.8 C LTR 0.74 20.6 C - -		SB	LTR			F E	LTR			F*	<u>LTR</u>	<u>1.11</u>	<u>74.1</u>	Ē	
36th Avenue EB LTR 0.79 38.1 D LTR 0.84 42.0 D Image: Constraint of the state of the			ection			F	-	1.20+		F	≣	<u>1.04</u>	<u>55.9</u>	Ē	-Shift centerline 7 ft to the south and restripe WB approach from one 22-ft travel lane with parking to one 10-ft exclusive left-turn lane and one 19-ft travel lane with parking for 250 ft. -Modify signal timing: shift 2 s green time to the all red time for the EB/WB phase and shift 3 s green time from the LPI phase to the NB/SB phase [EB/WB green time shifts from 31 s to 29 s; NB/SB green time shifts from 69 s to 72 s; LPI phase shifts from 10 s to 7 s].
WB LTR 0.74 32.6 C LTR 0.80 35.8 D Image: Constraint of the state of the sta	8. 36TH AVENUE &														
31st Street NB LTR 0.73 20.8 C LTR 0.82 25.8 C Image: Constraint of the stress of the stres stress of the stress of the stress of the stress of t	36th Avenue														
SB LTR 0.73 20.2 C LTR 0.74 20.6 C												<u> </u>			
	31st Street										ļ	<u> </u>			-Mitigation not required.
															4

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action			With M	itigation		
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
					AM	Peak I	Hour (c	ontinued)						
9. 41ST AVENUE &					_				_					
41st Avenue	WB	LR	0.31	16.7	В	LR	0.34	17.1	В	LR	0.39	19.8	B	-Modify signal timing: shift 2.4 2 s green time from WB phase
Vernon Boulevard	NB	TR	0.72	15.0	В	TR	0.83	19.4	B	TR	0.77	14.9	B	to NB/SB phase [NB/SB green
	SB	LT	1.20+	110.2	F	LT	1.20+	157.1	F*	LT	1.18	96.7	F	time shifts from $\frac{31.8}{32}$ s to
	Overall Inters	ection	0.86	68.4	Е	-	0.94	93.0	F	-	0.91	59.2	Е	34.2 34 s; WB green time shifts from 19.8 20 s to 17.4 18 s].
10. 30TH AVENUE 8	21ST STREET													
30th Avenue	EB	LTR	0.82	56.4	Е	LTR	0.82	56.4	E					
	WB	LTR	0.94	72.2	E	LTR	0.95	75.2	E					
21st Street	NB	LTR	0.59 0.52	16.1 <u>14.6</u>	В	LTR	0.61 0.53	16.5 <u>14.8</u>	В					-No mitigation required.
	SB	LTR	1.09 1.00	64.9 31.3	E C	LTR	1.13 1.04	82.5 43.7	F* D					Unmitigatable Impact
	Overall Inters	ection	1.04 0.98	51.6 32.8	Đ C	-	1.07 1.01	62.0 39.9	E D					
11. BROADWAY & V	ERNON BOULE	EVARD	/ 11TH	STREET										
Park Entrance	EB	LTR	0.01	28.2 28.0	С	LTR	0.01	28.2 28.0	С					
Broadway	WB	LTR	1.24 1.02	146.1 <u>97.7</u>	F	LTR	1.30 <u>1.08</u>	175.4 <u>111.8</u>	F*					
		-	-	-	-	-	-	-	-					
Vernon Boulevard	NB	LT	0.30	8.4 <u>2.1</u>	А	LT	0.31	8.5 2.1	А					
		R	0.10	6.8 1.2	Α	R	0.11	6.8 1.2	Α					-Unmitigatable Impacts
	SB	LTR	1.56 1.20+	284.7 207.0	F	LTR	1.63 1.20+	318.0 247.9	F*					
11th Street	NB	LTR	0.43	4 <u>2.6</u> 43.7	D	LTR	0.43	4 <u>2.6</u> 43.7	D					
	Overall Inters	ection	1.20+	177.5 120.6	F	-	1.20+	200.9 143.0	F					

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action			With M	itigation	,	Zeu mitersections)
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
12. ASTORIA BOULE	EVARD / 27TH A	VENU	E/NEW					ontinued) EET						
Astoria Boulevard	EB	L	1.06 <u>0.98</u>	106.3 <u>83.6</u>	F	L	1.06 <u>0.98</u>	106.3 <u>83.6</u>	F	L	0.86 <u>0.95</u>	54.7 <u>74.5</u>	Ð <u>E</u>	<u>-Prohibit parking along the SB</u> approach for 100 ft from the intersection for the weekday AM and PM peak periods and along the NB approach for 100 ft from
		TR	1.20+	471.4 <u>443.9</u>	F	TR	1.20+	4 80.3 452.8	F*	TR	1.20+	305.9 <u>419.8</u>	F	the intersection for the weekday <u>PM peak period</u> -
	WB	L	1.05	81.9	F	L	1.05	81.9	F	L	1.04 <u>1.05</u>	67.4 <u>81.9</u>	E	-Shift centerline 2 ft to the east and restripe SB approach from one 11-ft shared left-through
		TR	0.98 0.95	57.7 <u>53.7</u>	E D	TR	0.99 0.97	60.7 56.0	Е	TR	0.96 <u>0.97</u>	53.7 <u>56.0</u> 188.6	Ð E	lane and one 19-ft shared through-right lane with parking to
21st Street	NB	LTR	1.20+	212.7 <u>178.5</u> 173.3	F	LTR	1.20+	234.1 <u>198.7</u> 200.5	F*	LTR LTR	1.20+ 1.20+	<u>188.6</u> <u>150.9</u> 164.6	F F	one 11-ft shared left-through lane, one 10-ft travel lane, and one 11-ft parking which would
	SB	LTR	1.20+	<u>173.3</u> <u>172.0</u>	F	LTR	1.20+	<u>199.2</u>	F*	LT	<u>0.97</u>	<u>36.8</u>	D	serve as a right turn lane during
			-	-	-	-	-	-	-	<u>R</u>	0.66	<u>28.1</u>	<u>C</u>	the weekday AM and PM peak periods.
13. HOYT AVENUE N	Overall Inters			217.4 203.2	F	-	1.20+	232.1 217.8	F	-	1.20+	169.6 <u>144.2</u>	F	-Modify signal timing: shift 1 s green time from the NB/SB phase to the EB phase [EB phase green time shifts from 25 s to 26 s; NB/SB green time shifts from 50 s to 49 s; WB green time remains the same], -Modify signal phasing: Add a new lag phase for the EB/WB. The existing signal phasing [WB has 30 s green time; EB has 25 s green time; NB/SB has 50 s green time; would be modified to have the following: EB/WB will have 31 s green time; EB/WB will have 31 s green time; EB/WB will have 33 s green time; NB/SB will have 33 s green time; each phase will have 3 s amber and 2 s all red].
Hoyt Avenue North	EB	L	0.02	40.4	D	L	0.02	40.4	D					
		R	0.39	48.2	D	R	0.39	48.2	D					
	WB	L	1.10	91.1	F	L	1.16	116.3	F*					
		TR	0.26	15.0	В	TR	0.26	15.0	В					-
21st Street	NB	L	0.36	35.0	С Ф	L	0.37	36.2	D					-Unmitigatable Impacts
		Т	1.20+	188.8	F	Т	1.20+	191.5	F*					
	SB	TR	1.13	585.7 <u>99.5</u>	F	TR	1.16	600.0+ <u>112.2</u>	F*					
	Overall Inters		1.04	218.6 <u>108.3</u>	F	-	1.06	259.5 <u>122.3</u>	F					
14. HOYT AVENUE S Hoyt Avenue South	EB	SIREE	0.36	32.5	С	1	0.36	32.5	С	LTR	0.95	45.3	D	-Restripe EB approach of
	ED	TR	1.20+	32.5 219.6	F	TR	1.20+	32.5 219.6	F	LIR -	-	-	-	Hoyt Avenue South from one
21st Street	NB SB	LTR LTR	0.80	4 <u>5.0</u> 23.0 147.4	D C F	LTR LTR	0.83	54.1 <u>25.1</u> 170.0	<u>D*</u> <u>C</u> F*	LTR LTR	0.78 1.20+	40.0 20.3 138.1	D C F	11-ft exclusive left-turn lane and one 11-ft shared through-right lane to two 11-
	Overall Interse		1.20+	129.3 <u>123.7</u>	F	-	1.20+	143.6 136.3	F	-	1.16	92.7 <u>87.8</u>	F	ft shared left-through-right lanes. -Modify signal timing: shift 3 s green time from EB phase to NB/SB phase [EB green time shifts from 37 s to 34 s; NB/SB green time shifts from 73 s to 76 s].

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			N-	A			14/241-	A		· .	A/241- B.A	, c		ized intersections)
			<u>NO</u>	Action	1		<u>with</u>	Action	1	<u>'</u>		tigation	1	
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
							y Peak	Hour						
5. ROOSEVELT ISLA	ND BRIDGE / 3	6TH A	/ENUE	& VERNO	N BOU	LEVAF	RD	-						
Roosevelt Island Bridge	EB	L	0.24	12.5	В	L	0.37	14.7	В	Ē	<u>0.48</u>	<u>26.9</u>	<u>C</u>	
		TR	0.44	14.6	В	TR	0.68	20.0	В	<u>TR</u>	<u>0.77</u>	<u>35.2</u>	<u>D</u>	
36th Avenue	WB	LTR	0.37	14.0	В	LTR	0.59	18.2	В	<u>LTR</u>	<u>0.81</u>	<u>42.3</u>	<u>D</u>	-Modify the cycle length from
Vernon Boulevard	NB	LTR	1.04 0.91	54.5 27.5	₽ C	LTR	1.20+	219.6 <u>140.4</u>	F*	<u>LTR</u>	<u>0.96</u>	<u>38.0</u>	<u>D</u>	60 s to 90 s. EB/WB green time is 33 s; NB/SB green
	SB	LTR	0.85	27.4	С	LTR	0.93	36.0	D	<u>LTR</u>	<u>0.75</u>	22.3	<u>C</u>	time is 47 s; each phase has 3 s of amber and 2 s of red
	Overall Inters	ection	0.74 0.67	31.4 22.6	С	-	1.06 0.97	82.1 58.9	F	Ξ	<u>0.90</u>	<u>33.1</u>	<u>C</u>	time. Unmitigatable Impact
6. 36TH AVENUE & 2	1ST STREET													
36th Avenue	EB	LTR	0.89	56.6	E	LTR	1.20+	193.8	F*	L	0.72	51.2	D	-Shift centerline 6 ft to the
		-	-	-	-	-	-	-	-	TR	0.68	42.8	D	north and restripe EB
	WB	LTR	0.96	63.8	E	LTR	1.02	78.2	E*	L	0.38	36.8	D	approach from one 25-ft travel
		-	-	-	-	-	-	-	-	TR	0.84	50.8	D	lane to 11-ft exclusive left-turn
21st Street	NB	LTR	0.75	19.4	В	LTR	1.02	52.1	D*	LTR	0.99	41.3	D	lane, one 20-ft shared
	SB	LTR	0.69	18.1	В	LTR	0.73	19.3	В	LTR	0.71	17.5	В	through-right lane with parking for 200 ft.
	Overall Inters	ection	0.82	28.5	С	-	1.12	61.7	E	_	0.94	34.4	с	-Shift centerline 6 ft to the south and restripe WB approach from one 25-ft travel lane to 11-ft exclusive left-turn lane, one 20-ft shared through-right lane with parking for 125 ft. '-Modify signal timing: shift 2 s green time from EB/WB phase to NB/SB phase [EB/WB] green time shifts from 37 s to 35 s; NB/SB green time shifts from 73 s to 75 s].

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action		V	Vith M	itigation	~	
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
7. BROADWAY & 21	ST STREET				Midd	ay Pea	K HOUR	(continue	a)					
			1.20+	191.5	F	1.70	1.20+	227.0	F*	L	0.19	<u>37.8</u>	D	-Prohibit parking along the
Broadway	EB	LTR	<u>1.04</u>	<u>86.9</u>	F	LTR	<u>1.08</u>	<u>99.6</u>	F.,	_				EB approach for 200 ft from
		-	-	-	-	-	-	-	-	<u>TR</u>	<u>0.88</u>	<u>54.9</u>	D	the intersection and the EB receiving side for 250 ft from
	WB		1.20+	161.7	F	LTR	<u>1.28</u>	179.6	F*	L	0.77	65.9	E	the intersection.
	VV B	LTR	1.20+	<u>156.5</u>	Г		<u>1.20+</u>	<u>176.1</u>	F					-Shift centerline 3 ft to the
		-	-	-	-	-	-	-	-	<u>TR</u>	<u>0.84</u>	<u>51.9</u>	D	north and restripe EB approach from one 22-ft
21st Street	NB	LTR	0.90	28.3	С	LTR	0.93	30.9	С	LTR	<u>0.83</u>	<u>22.4</u>	C	travel lane with parking to
		LIIX	0.85	24.7	-	LIIK	0.87	<u>26.0</u>	-					one 10-ft exclusive left-turn
	SB	LTR	0.87 0.08	27.4 23.1	С	LTR	0.94 0.83	34.3 24.8	С	<u>LTR</u>	<u>0.80</u>	<u>21.5</u>	<u>C</u>	lane and one 15-ft travel lane for 200 ft.
8. 36TH AVENUE & 3	Overall Inters	ection	1.03 0.96	63.3 <u>46.1</u>	Е <u>D</u>	-	1.08 0.99	73.0 50.8	Е <u>D</u>	Ē	<u>0.84</u>	<u>29.3</u>	<u>C</u>	<u>-Shift centerline 7 ft to the</u> <u>south and restripe WB</u> <u>approach from one 22-ft</u> <u>travel lane with parking to</u> <u>one 10-ft exclusive left-turn</u> <u>lane and one 19-ft travel</u> <u>lane with parking for 250 ft.</u> <u>-Modify signal timing: shift 2</u> <u>s green time to the all red</u> <u>time for the EB/WB phase</u> <u>and shift 3 s green time from</u> <u>the LPI phase to the NB/SB</u> <u>phase [EB/WB green time</u> <u>shifts from 31 s to 29 s;</u> <u>NB/SB green time shifts from</u> <u>69 s to 72 s; LPI phase shifts</u> <u>from 10 s to 7 s]</u> <u>Unmitigatable Impacts</u>
36th Avenue	EB	LTR	0.88	42.3	D	LTR	0.93	48.3	D*	LTR	0.82	34.4	С	-Modify signal timing: shift 3
	WB	LTR	0.80	36.2	D	LTR	0.83	38.5	D	LTR	0.76	31.6	C	s green time from NB/SB
31st Street	NB	LTR	0.63	17.9	В	LTR	0.64	18.2	В	LTR	0.69	21.5	C	phase to EB/WB phase
	SB	LTR	0.53	15.3	В	LTR	0.53	15.4	В	LTR	0.56	17.9	В	[EB/WB green time shifts from 31 s to 34 s; NB/SB
	Overall Inters	ection	0.73	26.5	с	-	0.76	28.6	с	-	0.75	25.6	с	green time shifts from 49 s to 46 s].
9. 41ST AVENUE & V		-										-		
41st Avenue	WB	LR TR	0.21	15.5 15.7	B	LR TR	0.21	15.5 17.8	B	LR TR	0.23	17.7 14.0	B	 Mitigation not required. Modify signal timing: shift
Vernon Boulevard	NB SB	LT	0.74	15.7	B	LT	0.80	17.8	B	LT	0.74	14.0	B	2.4 2 s green time from WB
	Overall Inters		0.53	15.5	в	-	0.57	17.8	в	-	0.57	14.3	в	phase to NB/SB phase [NB/SB green time shifts from 31.8 <u>32</u> s to 34.2 <u>34</u> s; WB green time shifts from <u>19.8 20</u> s to <u>17.4 18</u> s]. [Measures reflect improvements needed for the weekday AM and PM peak periods.]
10. 30TH AVENUE &			•				•			•	<u> </u>		·	
30th Avenue	EB	LTR	0.50	38.7	D	LTR	0.50	38.7	D	-	-	-	-	
21 at Straat	WB NB	LTR LTR	0.65	45.0 23.4	D C	LTR LTR	0.66	45.2 25.7	D C	-	-	-	-	Mitigation not required
21st Street	SB	LTR	0.84	23.4	C	LTR	0.88	25.7	C	-	-	-	-	-Mitigation not required.
	Overall Inters		0.79	26.3	Č	-	0.81	28.1	Č	-	-	-	-	

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action			With M	itigation		
Intersection	Approach	Mvt.	v/c	Control Delay	LOS	Mvt.	v/c	Control Delay	LOS	Mvt.	v/c	Control Delay	LOS	Mitigation Measure
Intersection	Approach	WVt.	V/C	Delay				continue		WIVt.	V/C	Delay	L03	mitigation measure
11. BROADWAY &	ERNON BOULE	VARD	/ 11TH	STREET	Miuu	ayrea	Tiour	(continue	u)					
Park Entrance	EB	LTR	0.02	26.2 27.0	С	LTR	0.02	26.2 27.0	С					
Broadway	WB	LTR	1.09 1.03	93.0 100.7	F	LTR	1.13 1.07	106.3 119.4	F*					
		-	-	-	-	-	-	-	-					
Vernon Boulevard	NB	LT	0.31	8.8 <u>3.3</u>	А	LT	0.32	8.9 <u>3.6</u>	А					-Unmitigatable Impact
		R	0.20	7.8 <u>3.4</u>	A	R	0.21	7.9 <u>3.6</u>	А					-Oninitigatable impact
	SB	LTR	0.75 0.85	34.5 <u>46.4</u>	С Д	LTR	0.78 0.88	36.2 50.3	D					
11th Street	NB	LTR	0.26	33.6 <u>35.2</u>	¢ D	LTR	0.26	33.6 <u>35.6</u>	С <u>D</u>					
	Overall Inters		0.90 =	4 <u>1.0</u> <u>43.6</u>	D	-	0.93 E	4 5.1 <u>49.7</u>	D					
12. ASTORIA BOUL	EVARD / 27TH A	VENUE	/ NEW	TOWN A	VENUE	& 21S	T STRE	ET					_	
Astoria Boulevard	EB	L	0.36	36.8	D	L	0.36	36.8	D	L	0.33 <u>0.38</u>	25.3 <u>38.8</u>	С <u>D</u>	-Shift centerline 2 ft to the east and restripe SB approach
		TR	0.70	44.2	D	TR	0.73 0.72	45.0 <u>44.3</u>	D	TR	0.78 0.76	4 8.8 <u>47.9</u>	D	from one 11-ft shared left- through lane and one 19-ft
	WB	L	0.92	59.6	Е	L	0.92	59.6	Е	L	0.91 <u>0.92</u>	54.8 <u>59.6</u>	Ð E	shared through-right lane with parking to one 11-ft shared left-through lane, one 10-ft
		TR	0.71 <u>0.55</u>	44.2 <u>38.2</u>	D	TR	0.57	38.5 <u>38.4</u>	D	TR	0.61 <u>0.57</u>	4 0.7 <u>38.4</u>	D	travel lane, and one 11-ft parking which would serve as
21st Street	NB	LTR	1.20+	443.0	F	LTR	1.20+	501.8	F*	LTR	1.18 <u>1.20+</u>	116.2 <u>419.0</u>	F	a right turn lane during the weekday AM and PM peak
	SB	LTR	1.20+	220.2	F	LTR	1.20+	242.7	F*	LTR	1.03 <u>1.20+</u>	56.3 187.2	E E	<u>periods.</u> -Modify signal timing: shift 2 s
	Overall Inters	ection	1.19	205.0 205.7	F	-	1.20+	229.1 229.8	F	-	1.05 <u>1.20+</u>	67.5 <u>190.5</u>	E E	green time from the EB phase to the NB/SB phase [EB phase green time shifts from 34 s to 32 s; NB/SB green time shifts from 37 s to 39 s; WB green time remains the same]. -Modify signal phasing: Add a new lag phase for the EB/WB. The existing signal phasing (WB has 34 s green time; NB/SB has 37 s green time; NB/SB will have 51 s green time; NB/SB will have 51 s green time [each phase will have 3 s amber and 2 s all red].

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action		1	With M	itigation	<u> </u>	,
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mitigation Measure
13. HOYT AVENUE N	NORTH & 21ST	STREE	г		Midd	ay Pea	k Hour	(continue	a)					
Hoyt Avenue North	EB	L				L	0.12	42.3	D					
Hoyt Avenue North	LD		0.12	42.3	D			-						
		R	0.14	42.7	D	R	0.14	42.7	D					
	WB	L	0.91	49.9	D	L	0.96	55.7	E*					
		TR	0.17	14.3	в	TR	0.17	14.3	В					
21st Street	NB	L	0.13	25.6	с	L	0.13	25.6	С					
		Т	0.90	55.3	Е	т	0.90	56.2	Е					
	SB	TR	0.65	35.8	D	TR	0.66	36.1	D					
	Overall Interse	ection	0.74	45.7	D	-	0.76	48.9	D					-Unmitigatable Impact
14. HOYT AVENUE	SOUTH & 21ST	STREE	Г		·	·	·	•	·	·	· · · · · ·		·	
Hoyt Avenue South	EB	L	0.28	32.7	С	L	0.28	32.7	С	LTR	0.45	34.8	С	[Measures reflect
		TR	0.60	40.7	D	TR	0.60	40.7	D	-	-	-	-	improvements needed for
21st Street	NB	LTR	0.52	14.6	В	LTR	0.53	14.8	B	LTR	0.53	14.8	В	the weekday AM and PM peak periods.]
	SB	LTR	0.77	20.2	С	LTR	0.80	21.1	С	LTR	0.80	21.1	С	-Restripe EB approach of
	Overall Inters	ection	0.71	21.4	с	-	0.73	21.9	с	-	0.68	21.4	с	Hoyt Avenue South from one 11-ft excl. left-turn lane and one 11-ft shared through- right lane to two 11-ft shared left-through-right lanes.
							Peak H	our						
5. ROOSEVELT ISLA	AND BRIDGE / 3	<u>6TH AV</u>	ENUE	& VERNC	N BOL	JLEVAF	RD	1	1	r .				
Roosevelt Island Bridge	EB	L	0.49	14.8	В	L	0.64	17.5	В	Ē	<u>0.63</u>	<u>22.8</u>	C	
		TR	0.64	16.4	В	TR	1.12	83.2	F*	<u>TR</u>	<u>1.05</u>	<u>64.5</u>	E	- Partially Mitigated
36th Avenue	WB	LTR	0.34	13.5	В	LTR	0.56	18.8	В	<u>LTR</u>	<u>0.51</u>	<u>22.2</u>	<u>C</u>	-Modify the cycle length from 60 s to 90 s. EB/WB green
Vernon Boulevard	NB	LTR	1.20+	236.2 233.8	F	LTR	1.20+	44 3.3 439.4	F*	LTR	<u>1.20+</u>	<u>439.1</u>	Ē	time is 40 s; NB/SB green time is 40 s; each phase has
	SB	LTR	1.06	63.3	Е	LTR	1.10	80.2	F*	<u>LTR</u>	<u>1.04</u>	<u>64.9</u>	Ē	3 s of amber and 2 s of red time.
	Overall Inters	ection	1.07 1.07	108.6 107.7	F	-	1.20+	185.0 183.8	F	=	<u>1.20+</u>	<u>175.9</u>	Ē	Unmitigatable Impacts
6. 36TH AVENUE & 2	21ST STREET			10/11				100.0						
36th Avenue	EB	LTR	0.62	37.4	D	LTR	1.20+	250.7	F*	L	0.70	41.9	D	-Shift centerline 6 ft to the
		-	-	-	-	-	-	-	-	TR	0.74	40.3	D	north and restripe EB
	WB	LTR	0.89	54.2	D	LTR	0.99	70.8	E*	L	0.36	34.9	С	approach from one 25-ft travel lane to 11-ft exclusive
		-	-	-	-	-	-	-	-	TR	0.75	43.7	D	left-turn lane, one 20-ft
21st Street	NB SB	LTR LTR	1.03	44.7 22.8	D C	LTR LTR	1.03 0.85	44.9 24.0	D C	LTR LTR	1.03	44.9 24.0	D C	shared through-right lane
	Overall Inters		0.98	38.0	D	-	1.17	66.0	E	-	0.93	37.6	D	with parking for 200 ft. -Shift centerline 6 ft to the south and restripe WB approach from one 25-ft travel lane to 11-ft exclusive left-turn lane, one 20-ft shared through-right lane with parking for 125 ft.

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action			With M	itigation	,	
				Control				Control				Control		
Intersection	Approach	M∨t.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
	OT OTDEET				PM	Peak I	lour (c	ontinued)						
7. BROADWAY & 21	SISIREEI	1		293.1	1			339.4			0.00	44.0		-Prohibit parking along the
Broadway	EB	LTR	1.20+	283.2	F	LTR	1.20+	334.0	F*	F	<u>0.60</u>	<u>41.9</u>	D	EB approach for 200 ft from
		-	-	-	-	-	-	-	-	<u>TR</u>	<u>0.88</u>	<u>46.7</u>	<u>D</u>	the intersection and the EB receiving side for 250 ft from
	WB	LTR	1.20+	313.1 301.3	F	LTR	1.20+	333.3 321.5	F*	Ŀ	<u>1.05</u>	<u>89.1</u>	E	the intersection. -Shift centerline 3 ft to the
21st Street	NB	-	-	-	-	-	-	<u>-</u>	-	<u>TR</u>	<u>0.75</u>	<u>43.8</u>	D	north and restripe EB
		LTR	1.05 1.04	54.1 51.1	D	LTR	1.11 1.09	78.3 71.7	E*	<u>LTR</u>	<u>1.04</u>	<u>51.4</u>	<u>D</u>	approach from one 22-ft travel lane with parking to one 10-ft exclusive left-turn
			0.84	<u>25.2</u>	_		0.86	26.6	_	LTR	0.81	22.0	C	lane and one 15-ft travel
	SB	LTR	0.82	24.2	С	LTR	0.84	25.4	С		<u></u>	<u></u>	×	lane for 200 ft.
8. 36TH AVENUE & 3	Overall Inters	ection	1.20+ <u>1.20</u>	99.5 95.8	F	-	1.20+	118.7 113.7	F	Ξ	<u>1.05</u>	<u>41.2</u>	D	<u>Shift centerline 7 ft to the</u> <u>south and restripe WB</u> <u>approach from one 22-ft</u> <u>travel lane with parking to</u> <u>one 10-ft exclusive left-turm</u> <u>lane and one 19-ft travel</u> <u>lane with parking for 250 ft.</u> <u>-Modify signal timing: shift 2 <u>s green time to the all red</u> <u>time for the EB/WB phase</u> <u>and shift 3 s green time from</u> <u>the LPI phase to the NB/SB</u> <u>phase [EB/WB green time</u> <u>shifts from 31 s to 29 s:</u> <u>NB/SB green time shifts from 69 s to 72 s; LPI phase shifts</u> <u>from 10 s to 7 s].</u> <u>Unmitigatable Impacts</u></u>
					_		1.00	80.1	-		0.95	4 2.5	_	
36th Avenue	EB	LTR	0.90	38.7	D	LTR	1.08	<u>81.2</u>	F*	LTR	0.96	43.0	D	-Modify signal timing: shift 3
	WB	LTR	0.80	35.9	D	LTR	0.83	37.4 37.9	D	LTR	0.75 0.76	30.8 31.1	С	s green time from NB/SB phase to EB/WB phase
31st Street	NB	LTR	0.83	25.2	С	LTR	0.84 0.83	25.8 25.2	С	LTR	0.89	32.9 31.9	С	[EB/WB green time shifts from 31 s to 34 s; NB/SB
	SB	LTR	0.56	16.1	В	LTR	0.56	16.2	В	LTR	0.60	18.9	В	green time shifts from 49 s to
								37.7	_		0.92	31.1	~	46 s].
	Overall Inters	ection	0.86	27.7	С	-	0.93	37.9	D	-	0.91	<u>31.0</u>	С	
9. 41ST AVENUE & \	ERNON BOUL													
41st Avenue	WB	LR	0.44	18.9	В	LR	0.52	20.4	С	LR	0.59	24.8	С	-Modify signal timing: shift
Vernon Boulevard	NB	TR	1.17	97.4	F	TR	1.20+	114.8	F*	TR	1.13	76.7	Е	2.4 2 s green time from WB
	SB	LT	1.14	86.1	F	LT	1.20+	164.2	F*	LT	1.11	70.5	Е	phase to NB/SB phase
	Overall Inters	ection	0.89	85.2	F	-	1.01	125.4	F	-	0.95	68.7	E	[NB/SB green time shifts from 31.8 <u>32</u> s to 34.2 <u>34</u> s; WB green time shifts from <u>19.8</u> <u>20</u> s to 17.4 <u>18</u> s].
10. 30TH AVENUE &				0										
30th Avenue	EB	LTR	0.52	39.2	D	LTR	0.53	39.5	D	-	-	-	-	
	WB	LTR	0.67	45.3	D	LTR	0.67	45.6	D	-	-	-	-	
21st Street	NB	LTR	0.96 <u>0.89</u>	31.2 24.2	С	LTR	1.01 <u>0.94</u>	42.4 28.3	D	-	-	-	-	-Mitigation not required.
	SB	LTR	0.70 0.65	18.1 <u>16.8</u>	В	LTR	0.72 <u>0.67</u>	18.6 <u>17.3</u>	В	-	-	-	-	
	Overall Inters	ection	0.86	28.0	С	Ι.	0.90 0.85	33.9 <u>26.3</u>	С	-	_	-	-	

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No	Action			With	Action			With M	itigation	<u>,</u>	,
Intersection	Approach	Mvt.	V/C	Control Delay	LOS	Mvt.	V/C	Control Delay	LOS	Mvt.	v/c	Control Delay	LOS	Mitigation Measure
intersection	Approach	WIVL.	10	Delay				ontinued		WIVL.	1/0	Delay	200	mitigation measure
11. BROADWAY & V	ERNON BOULE	VARD	/ 11TH	STREET										
Park Entrance	EB	LTR	0.03	33.2 <u>33.5</u>	С	LTR	0.03	33.2 <u>33.5</u>	с					
Broadway	WB	LTR	1.18 <u>1.09</u>	140.0 <u>134.4</u>	F	LTR	1.21 1.12	150.8 <u>157.1</u>	F*					
		-	-	-	-	-	-	-	-					
Vernon Boulevard	NB	LT	0.57	11.0 <u>3.4</u>	B A	LT	0.60	11.5 <u>4.1</u>	₽ A					
		R	0.16	6.6 <u>3.7</u>	A	R	0.19	6.7 <u>4.2</u>	A					-Unmitigatable Impacts
	SB	LTR	1.15 <u>1.20+</u>	117.2 243.5	F	LTR	1.20+	138.4 293.9	F*					
11th Street	NB	LTR	0.37	39.1 <u>41.9</u>	D	LTR	0.37	39.1 <u>41.9</u>	D					
	Overall Interse		1.17 	67.5 <u>95.5</u>	E E	-	1.20 ≞	74.5 114.0	E E					
12. ASTORIA BOUL	EVARD / 27TH A	VENU	E/NEW	TOWN A	VENUE	& 21S	T STRE	ET	1			40.0	1	Drobibit parking along the CD
Astoria Boulevard	EB	L	0.59	45.2	D	L	0.59	45.2	D	L	0.57	40.9 43.9	D	<u>-Prohibit parking along the SB</u> approach for 100 ft from the intersection for the weekday AM
		TR	1.20+	162.8 152.6	F	TR	1.28 <u>1.20+</u>	<u>180.2</u> <u>169.8</u>	F*	TR	1.09 1.20+	95.3 149.9	F	and PM peak periods and along the NB approach for 100 ft from
	WB	L	0.96 <u>0.95</u>	75.9 <u>73.1</u>	Е	L	0.96 0.95	75.9 <u>73.1</u>	E	L	0.95 0.91	72.6 <u>66.1</u>	Е	the intersection for the weekday PM peak period
		TR	1.15 <u>1.11</u>	127.3 <u>111.1</u>	F	TR	1.16 <u>1.12</u>	132.3 <u>115.9</u>	F*	TR	0.82 <u>1.07</u>	4 <u>5.7</u> 96.3	Ð E	- Restripe the NB approach from one 11-ft shared left-through
21st Street	NB	DefL	1.20+	526.1 <u>466.5</u>	F	DefL	2.08 <u>1.95</u>	526.1 <u>466.5</u>	F	LT	1.20+	524.1 235.2	F	lane and one 20-ft shared through-right lane with parking to
		TR	1.20+	434.7	F	TR	2.02 <u>1.20+</u>	492.4	F*	R	0.52	25.9	С	one 11-ft shared left-through lane, one 10-ft travel lane, and one 10-ft parking lane which
	SB	LTR	1.20+	250.0	F	LTR	1.52 <u>1.20+</u>	267.8	F*	LT	1.20+ 0.85	424.6 <u>34.1</u>	₽ <u></u>	would serve as a right turn lane during the weekday PM peak
			-	-	-	-	-	-	-	R	1.20+ <u>1.15</u>	214.0 <u>112.3</u>	F	period. -Shift centerline 2 ft to the east
	Overall Inters	ection	1.20+	254.7 249.9	F	-	1.20+	280.9 <u>276.3</u>	F	-	1.20+	220.6 <u>118.7</u>	F	and restripe SB approach from one 11-ft shared left-through lane and one 19-ft shared through-right lane with parking to one 11-ft shared left-through lane, one 10-ft travel lane, and one 11-ft parking lane which would serve as a right turn lane during the weekday AM and PM peak periods. -Modify signal timing: shift 1.s green time from the NB/SB phase to the EB phase and 1.s green time from the NB/SB phase to the EB phase and 1.s green time shifts from 24 s to 25 s; EB phase green time shifts from 28 s to 29 s; NB/SB green time shifts from 53 s to 52 s]. The existing signal phasing IWB has 24 s green time; EB MBS has 53 s green time; NB/SB has 53 s green time; NB/SB has 53 s green time; NB/SB will have 33 s green time; EB/MB exclusive left- turn phase will have 15 s green time; NB/SB will have 33 s green time; NB/SB will have 35 s green time for the have 35 s green time fo

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No Action			With Action			With Mitigation					
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
			_		PM	Peak I	lour (c	ontinued)						
13. HOYT AVENUE N	IOR I H & 215 I	SIREE	1			1	1			-	1			
Hoyt Avenue North	EB	L	0.10	41.9	D	L	0.10	41.9	D					
		R	0.18	43.3	D	R	0.18	43.3	D					
	WB	L	1.07	86.0	F	L	1.10	97.2	F*					
		TR	0.30	15.9	В	TR	0.30	15.9	В					
21st Street	NB	L	0.21	26.9	С	L	0.21	26.9	С					-Unmitigatable Impacts
		Т	1.20+	166.0	F	т	1.20+	176.4	F*					
	SB	TR	0.87	580.5 4 5.6	₽ D	TR	0.87	600.0+ <u>45.9</u>	<mark>F</mark> * D					
	Overall Intersection		0.98	191.2 95.0	F	-	1.00	212.9 103.0	F					
14. HOYT AVENUE S	OUTH & 21ST	STREE	Т											•
Hoyt Avenue South	EB	L	0.24	31.7	С	L	0.24	31.7	С	LTR	0.68	40.8	D	-Restripe EB approach of
		TR	0.99	72.1	Е	TR	0.99	72.1	E	-	-	-	-	Hoyt Avenue South from one
21st Street	NB	LTR	1.20+	286.8 171.7	F	LTR	1.20+	325.9 <u>196.0</u>	F*	LTR	1.20+	279.9 157.7	F	11-ft exclusive left-turn lane and one 11-ft shared
	SB	LTR	1.20+	185.2	F	LTR	1.20+	204.9	F*	LTR	1.20+	167.6	F	through-right lane to two 11-
	Overall Interse		1.20+	202.3 <u>160.9</u>	F	-	1.20+	226.8 <u>179.5</u>	F	-	1.12	188.6 <u>144.2</u>	Ŀ	ft shared left-through-right lanes for 250 ft. -Modify signal timing: shift 3 s green time from EB phase to NB/SB phase [EB green time shifts from 37 s to 34 s; NB/SB green time shifts from 73 s to 76 s].

2) Overall intersection V/C ratio is the critical lane groups' V/C ratio. Denotes a significant impact.

TRANSIT

As discussed in Chapter 14, "Transportation," the proposed project would not result in any significant adverse subway station or tramway impacts in either the 2018 or 2038 analysis year. However, it would result in significant adverse impacts to bus line-haul levels for the Q102 bus and the Red Bus. In the eastbound and westbound directions the Q102 bus route would experience significant adverse impacts during the PM peak period in the 2018 analysis year and during both the AM and PM peak period in the 2038 analysis year. The Red Bus route would also result in significant adverse impacts to bus line-haul levels for the southbound direction in the AM peak period and the northbound direction in the PM peak period during the 2038 analysis year. Potential measures to mitigate these significant adverse impacts are described below.

BUS LINE HAUL

The proposed project would result in significant adverse bus line-haul impacts on the Q102 route under 2018 Phase 1 and to both the Q102 and Red Bus routes under 2038 Full Build conditions. Under Phase 1-2018, during the PM peak period, the eastbound Q102 would exceed the NYCT guideline capacity. Under Full Build-2038, during the AM and PM peak periods, both the eastbound and westbound Q102 would exceed the guideline capacity while the Red Bus would exceed the RIOC guideline capacity in the southbound and northbound directions during the AM and PM peak periods.

Table 22-4 provides comparisons of existing service and the number of buses required to fully mitigate the identified significant adverse line-haul impacts along the Q102 bus route under Phase 1 and the Q102 and Red Bus route under Full Build of the project. The Full Build-2038 mitigation accounts for all buses needed to accommodate the 2038 projected passenger volumes independent of the Phase 1-2038 mitigation. NYCT and RIOC routinely monitors changes in bus ridership and makes the necessary service adjustments where warranted.

Table 22-4 Mitigated Future With Action Condition (Capacity Improvement): Bus Line Haul Levels													
Analysis		Peak	Eastbound/N Buses pe	er Hour	Westbound/Southbound Buses per Hour								
Year	Route	Period	Existing	Mitigation	Existing	Mitigation							
2018	Q102	AM	4	n/a	3	n/a							
2010	QTUZ	PM	2	3	2	n/a							
2038	Q102	AM	4	6	3	5							
2036	QTUZ	PM	2	7	2	6							
2020	Red	AM	8	n/a	8	10							
2038	Bus	PM	8	9	8	n/a							

. . .

PEDESTRIANS

PHASE 1-2018 ANALYSIS YEAR (2018 WITH ACTION CONDITION)

The proposed project would not result in any significant adverse impacts on pedestrian operations.

Notes: The Q102 bus route operates standard buses with a guideline capacity of 54 passengers per bus and the Red Bus route operates with a guideline capacity of 55 passengers per bus.

FULL BUILD-2038 ANALYSIS YEAR (2038 WITH ACTION CONDITION)

Under Full Build-2038, the proposed project would result in significant adverse pedestrian impacts at the following locations on West Road and West Main Street:

- West Road: The east sidewalk between West Main Street and the subway station; and
- West Main Street: The east sidewalk between the Tram Station West bus stop and the Queensboro Bridge.

West Road between West Main Street and Subway Station

At this location, the east sidewalk would experience the following changes:

- LOS B (1.65 PMF) under the No Action condition to LOS D (9.28 PMF) under the With Action condition during the AM peak period;
- LOS B (1.01 PMF) under the No Action condition to LOS D (7.06 PMF) under the With Action condition during the midday peak period; and
- LOS B (2.72 PMF) under the No Action condition to LOS D (11.48 PMF) under the With Action condition during the PM peak periods.

The significant adverse impacts at this sidewalk would be fully mitigated by widening its existing width of 6.4 feet to 8.9 feet, thereby increasing its effective width from 2.7 feet to 5.2 feet.

West Main Street between the Tram Station West Bus Stop and Queensboro Bridge

At this location, the east sidewalk would experience the following changes:

- LOS B (1.20 PMF) under the No Action condition to LOS D (7.06 PMF) under the With Action condition during the AM peak period; and
- LOS B (1.78 PMF) under the No Action condition to LOS D (8.52 PMF) under the With Action condition during the PM peak period.

The significant adverse impacts at this sidewalk would be fully mitigated by widening its existing width of 6.4 feet to 8.0 feet, thereby increasing its effective width from 3.6 feet to 5.2 feet. The measures described above, which have been determined to be feasible, and the mitigated conditions are summarized in **Table 22-5**. In the event the proposed sidewalk widening is determined to be infeasible, the projected impacts would be deemed unmitigatable.

Table 22-5 2038 No Action, With Action, and Mitigated Conditions Pedestrian Level of Service Analysis

Location	Mitigation Measures	Existing Effective Width (ft.)				ction	Proposed Effective Width (ft.)	Mitig	
	Weekday PM Peak	15-Minutes							
West Road, between West Main Street and the Subway Station- East Sidewalk	Sidewalk widening by 2.5 feet	2.7	2.72	В	11.48	D	5.2	5.96	С
West Main Street between the Tram Station West Bus Stop and Queensboro Bridge- East Sidewalk	0	3.6	1.78	В	8.52	D	5.2	5.90	С
Note: PMF = pedestrians per minute per foot.									

EFFECTS OF TRAFFIC MITIGATIONS ON PEDESTRIAN OPERATIONS

As previously described, intersection operations would be improved with the implementation of the recommended traffic mitigation measures. These measures would include changes to existing signal timings, installation of new signals, and modifications to lane utilization. A review of the effects of these changes on pedestrian circulation and levels of service showed that they would not alter the conclusions made for the pedestrian impact analyses, nor would they result in the potential for any additional significant adverse pedestrian impacts.

At the newly signalized (per traffic mitigation) intersection of Main Street and West Road, a pedestrian crosswalk analysis was conducted to determine if the proposed intersection reconfiguration and signal timing would continue to adequately accommodate pedestrian crossing at the intersection. As shown in **Table 22-6**, the traffic mitigation measures recommended for this intersection would not result in any significant adverse pedestrian impacts.

MITIGATION IMPLEMENTATION

Subject to approvals of the relevant agencies, including NYCDOT, RIOC, and NYCT, the recommended mitigation measures would be implemented to mitigate the projected significant adverse transportation impacts at the completion of the project's Phase 1-2018 and Full Build-2038 conditions. <u>However, between Phase 1-2018 and Full Build-2038 conditions</u>, <u>Cornell will</u>

coordinate the implementation schedule for traffic mitigation measures shown above for 2038 conditions with RIOC and NYCDOT.

		Street	Crosswalk Width	Conditions with conflicting vehicles							
		Width		AM		Midday		Р	М		
Location	Crosswalk	(feet)	(feet)	SFP	LOS	SFP	LOS	SFP	LOS		
Main Street	West	27.5	12.0	49.9	В	40.0	С	39.1	С		
and West Road	Northeast	35.5	12.0	104.5	А	212.6	А	206.8	Α		
Note: SFP = square feet per pedestrian.											

Table 22-6 2038 With Action Condition Crosswalk Analysis with Traffic Mitigation

D. CONSTRUCTION

The analysis undertaken in Chapter 20, "Construction," concludes that the proposed project would result in significant adverse construction impacts related to transportation and noise (i.e., construction noise impacts on open space).

TRANSPORTATION

During Phase 1 construction of the proposed project, significant adverse impacts are expected for traffic and transit conditions. During Phase 2 construction, significant adverse impacts are expected for traffic, transit, and pedestrian conditions. These findings are summarized below.

TRAFFIC

Four intersections (of the seven analyzed) would experience significant adverse traffic impacts during Phase 1 construction. Impacts at <u>three of the two of the four</u> intersections could be mitigated using standard mitigation measures typically implemented by NYCDOT. <u>Significant impacts at one location could only be partially mitigated</u>. These measures would also be consistent with <u>similar to</u> those proposed to mitigate the intersection impacts associated with the project's build-out and occupancy. Two impacts are currently identified as unmitigatable, but additional review of potential mitigation measures will be undertaken for the Final EIS that may fully or partially mitigate these significant impacts.

For Phase 2 construction, the cumulative operational and construction traffic would be of lower magnitudes than what the overall project would generate when completed in 2038. Therefore, potential traffic impacts during peak Phase 2 construction would be within the envelope of significant adverse traffic impacts identified for the 2038 With Action condition in Chapter 14, "Transportation," and mitigatable and unmitigatable impacts identified above would apply to Phase 2 construction conditions as well. The required mitigation measures for those locations that could be mitigated are expected to be part of those presented for the 2038 full build-out of the proposed project. These mitigation measures could be implemented at the discretion of NYCDOT during construction of Phase 2.

TRANSIT

During construction of Phase 1, because most construction workers parking at the Motorgate garage would rely on the Red Bus for travel to/from the project site, during off-peak hours when the Red Bus operates at comparatively lower frequencies, there is a potential for a line-haul

impact on the Red Bus that would warrant an increase in its service during off-peak hours (i.e., three additional buses during the 6 to 7 AM and 3 to 4 PM construction peak hours). <u>Cornell has committed to fund the operating costs associated with providing additional Red Bus service if project activity adversely impacts the Red Bus service during the construction period.</u>

A significant adverse impact has been identified for the Q102 bus route due to the projected increase in demand from the completed buildings, and this impact would continue during the Phase 2 construction period. Mitigation measures identified above for the operational impact would be proposed to mitigate the construction-period impact.

PEDESTRIANS

Pedestrian trips generated by construction workers are not expected to result in significant adverse pedestrian impacts during Phase 1 construction. After the completion of the Phase 1 and Phase 2A components of the proposed project, the combination of the Phase 2 construction worker pedestrian trips with those generated by the completed Phase 1 and Phase 2A buildings during the commuter peak hours may result in similar significant adverse pedestrian impacts as those discussed in Chapter 14, "Transportation," and may warrant the earlier implementation of the recommended sidewalk widening described above. In the event the widening is determined to be infeasible, the projected impacts would be deemed unmitigatable.

NOISE IMPACTS ON OPEN SPACE

The proposed project would result in significant adverse impacts with respect to construction noise, as follows:

- During construction of Phase 1, the open space areas along Main Street would experience exceedances due to trucks and workers travelling on Main Street to and from the project site during the AM construction traffic peak hour (6 to 7 AM);
- During construction of Phase 2, South Point Park and the waterfront promenades on the east and west sides of the Island adjacent to the project site would experience noise levels in the mid to high 70s of dBA for over 24 months. These exceedances would be due to the operation of on-site construction equipment.

No practical and feasible mitigation measures have been identified that could be implemented to reduce noise levels to below the 55 dBA $L_{10(1)}$ guideline within the impacted open space areas (i.e., the open spaces along Main Street, the waterfront promenade, or South Point Park). Noise levels in these spaces would exceed the 55 dBA $L_{10(1)}$ noise level recommended for outdoor areas requiring serenity and quiet by the *CEQR Technical Manual* noise exposure guidelines. However while the 55 dBA $L_{10(1)}$ guideline is a worthwhile goal for outdoor areas requiring serenity and quiet, due to the level of activity present at most New York City open space areas and parks (except for areas far away from traffic and other typical urban activities) this relatively low noise level is often not achieved. For example, existing noise levels at the waterfront promenade and South Point Park are already above the 55 dBA $L_{10(1)}$ guideline due to noise from vehicular traffic on the Queensboro Bridge and on the FDR Drive. To achieve noise levels that would meet the 55 dBA $L_{10(1)}$ guideline, measures would need to be implemented to control noise from the Queensboro Bridge; the implementation of such barriers on the bridge would not be possible because of the bridge's landmarked status.